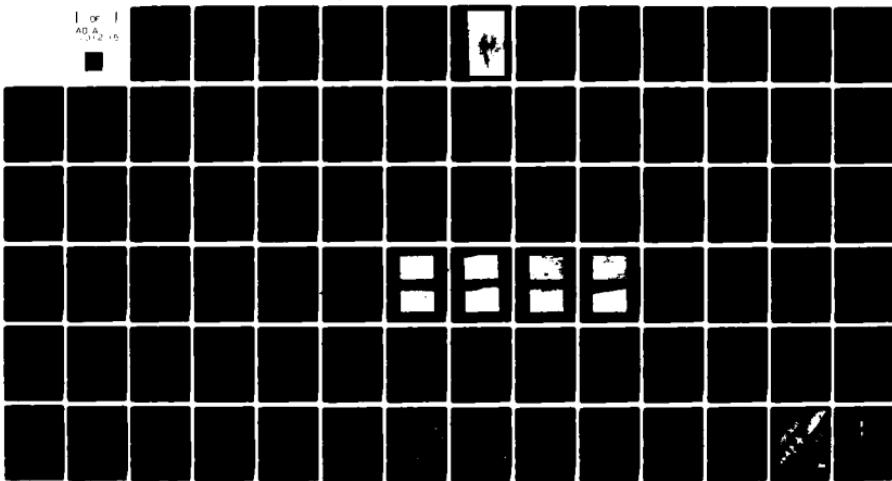


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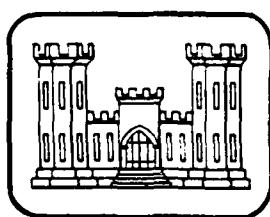
BRESKIN POND DAM No. 1

NDI No. PA 01141

PennDER No. 65-141

Dam Owner: Joseph Breskin

PHASE I INSPECTION REPORT
NATIONAL DAM INSPECTION PROGRAM



prepared for

DEPARTMENT OF THE ARMY
Baltimore District, Corps of Engineers
Baltimore, Maryland 21203

prepared by

MICHAEL BAKER, JR., INC.

Consulting Engineers
4301 Dutch Ridge Road
Beaver, Pennsylvania 15009

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OHIO RIVER BASIN

BRESKIN POND DAM ~~NO. 1~~
WESTMORELAND COUNTY, COMMONWEALTH OF PENNSYLVANIA
(NDI ~~NO.~~ PA-01141
PennDER ~~NO.~~ 65-141)

PHASE I INSPECTION REPORT
NATIONAL DAM INSPECTION PROGRAM

Prepared for: DEPARTMENT OF THE ARMY
Baltimore District, Corps of Engineers
Baltimore, Maryland 21203

Prepared by: MICHAEL BAKER, JR., INC.
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Beaver, Pennsylvania 15009
Contract/DACW31-81-C-0011

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PREFACE

This report is prepared under guidance contained in the "Recommended Guidelines for Safety Inspection of Dams," for Phase I Investigations. Copies of these guidelines may be obtained from the Office of Chief of Engineers, Washington, D.C. 20314. The purpose of a Phase I Investigation is to identify expeditiously those dams which may pose hazards to human life or property. The assessment of the general condition of the dam is based upon available data and visual inspections. Detailed investigation, and analyses involving topographic mapping, subsurface investigations, testing, and detailed computational evaluations are beyond the scope of a Phase I Investigation; however, the investigation is intended to identify any need for such studies.

In reviewing this report, it should be realized that the reported condition of the dam is based on observations of field conditions at the time of inspection along with data available to the inspection team. In cases where the reservoir was lowered or drained prior to inspection, such action, while improving the stability and safety of the dam, removes the normal load on the structure and may obscure certain conditions which might otherwise be detectable if inspected under the normal operating environment of the structure.

It is important to note that the condition of a dam depends on numerous and constantly changing internal and external conditions, and is evolutionary in nature. It would be incorrect to assume that the present condition of the dam will continue to represent the condition of the dam at some point in the future. Only through frequent inspections can unsafe conditions be detected and only through continued care and maintenance can these conditions be prevented or corrected.

Phase I Inspections are not intended to provide detailed hydrologic and hydraulic analyses. In accordance with the established guidelines, the spillway design flood is based on the estimated "Probable Maximum Flood" for the region (greatest reasonably possible storm runoff), or fractions thereof. The spillway design flood provides a measure of relative spillway capacity and serves as an aid in determining the need for more detailed hydrologic and hydraulic studies, considering the size of the dam, its general condition and the downstream damage potential.

PHASE I REPORT
NATIONAL DAM INSPECTION PROGRAM

Breskin Pond Dam No. 1, Westmoreland County, Pennsylvania
NDI No. PA 01141, PennDER No. 65-141
Unnamed Tributary to Fourmile Run
Inspected 5 December 1980

ASSESSMENT OF
GENERAL CONDITIONS

↓
Breskin Pond Dam No. 1, owned by Joseph Breskin, is classified as a "Significant" hazard - "Small" size dam. The dam was found to be in fair overall condition at the time of inspection.

Hydraulic/hydrologic evaluations, performed in accordance with procedures established by the Baltimore District, Corps of Engineers, for Phase I Inspection Reports, revealed that the spillway will pass approximately 30 percent of the Probable Maximum Flood (PMF) before overtopping will occur. A spillway design flood (SDF) in the range of the 100-year flood to the 1/2 Probable Maximum Flood (1/2 PMF) is required for Breskin Pond Dam No. 1. The 100-year flood was chosen because the dam is on the low end of the "Small" size category in terms of storage capacity. The total duration and maximum depth of overtopping during the 100-year flood are 0.6 hours and 0.34 foot, respectively. The spillway is therefore assessed as being "Inadequate." It is recommended that the owner, under the guidance of a professional engineer, develop remedial measures to ensure that the dam will not be overtopped by the 100-year flood.

The inspection and review of available information revealed certain items of work which should be performed without delay by the owner. Item 1 below should be completed under the guidance of a qualified professional engineer experienced in the design and construction of earth dams.

- 1) Develop remedial measures to ensure that the dam will not be overtopped by the 100-year flood.
- 2) Provide riprap or some means of protecting the emergency spillway channel from erosion during the SDF.

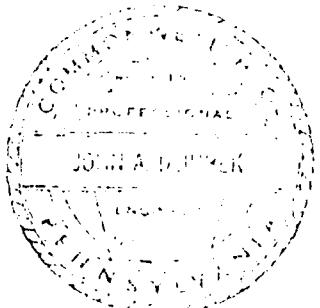
BRESKIN POND DAM No. 1

- 3) Cut the brush and saplings on the downstream slope.
- 4) Fill the rodent hole on the downstream face of the embankment.

A formal emergency warning system has been prepared. It is recommended that formal emergency operation procedures for the dam be prepared. It is further recommended that formal inspection, maintenance, and operation procedures and records be developed and implemented. These should be included in a formal maintenance and operations manual for the dam.

Submitted by:

MICHAEL BAKER, JR., INC.



John A. Dziubek, P.E.
Engineering Manager-Geotechnical

Date: April 24, 1981

Approved by:

DEPARTMENT OF THE ARMY
BALTIMORE DISTRICT, CORPS OF ENGINEERS

James W. Peck
JAMES W. PECK
Colonel, Corps of Engineers
District Engineer

Date: 11 May 81

BRESKIN POND DAM NO. 1



Overall View of Dam from Right Abutment

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- Appendix A - Visual Inspection Check List, Field Sketch, Top of Dam Profile, and Typical Cross-Section
- Appendix B - Engineering Data Check List
- Appendix C - Photograph Location Plan and Photographs
- Appendix D - Hydrologic and Hydraulic Computations
- Appendix E - Plates
- Appendix F - Regional Geology

PHASE I INSPECTION REPORT
NATIONAL DAM INSPECTION PROGRAM
BRESKIN POND DAM No. 1
NDI No. PA 01141, PennDER No. 65-141

SECTION 1 - PROJECT INFORMATION

1.1 GENERAL

- a. Authority - The Dam Inspection Act, Public Law 92-367, authorized the Secretary of the Army, through the Corps of Engineers, to initiate a program of inspection of dams throughout the United States.
- b. Purpose of Inspection - The purpose of the inspection is to determine if the dam constitutes a hazard to human life or property.

1.2 DESCRIPTION OF PROJECT

- a. Description of Dam and Appurtenances - Breskin Pond Dam No. 1 is an earthfill embankment with a height of 29 feet and a length of 338 feet. The embankment has a minimum crest elevation of 1491.8 feet Mean Sea Level (ft. M.S.L.), a crest width of 24 feet and side slopes of 2.25H:1V (Horizontal to Vertical) upstream and 2H:1V downstream.

The principal spillway of the dam consists of a 30-inch diameter bituminous coated corrugated metal pipe (BCCMP) which acts as a fixed crest riser. It is connected to a 24-inch BCCMP that extends through the embankment and exits near the toe. According to the owner, there are four corrugated metal anti-seep collars (6 ft. by 6 ft.) on this pipe. The crest of the principal spillway is at elevation 1488.5 ft. M.S.L. and is protected with a trash rack.

The emergency spillway, located on the right abutment, is an unvegetated (at time of inspection) earth trapezoidal channel. The spillway has a bottom width of 6 feet (perpendicular to flow) and side slopes of 5H:1V. The spillway crest elevation of 1489.9 ft. M.S.L. is about 2 feet lower than the minimum top of dam. The spillway empties into a grass-lined trapezoidal discharge channel.

There is an 8-inch BCCMP running through the bottom of the embankment that can be used to dewater the reservoir. This pipe is controlled by an Armco slide gate which is operated from the upstream crest of the embankment. According to the owner, there are four corrugated metal anti-seep collars (5 ft. by 5 ft.) on this pipe. There is also a leaf gate located at the outlet end of the pipe.

- b. Location - Breskin Pond Dam No. 1 is located on an unnamed tributary of Fourmile Run, approximately 3.8 miles north of Mansville, Pennsylvania. The structure is located in Ligonier Township, Westmoreland County, Pennsylvania. The coordinates of the dam are N 40° 13.9' and W 79° 19.3'. The dam and reservoir are shown on USGS 7.5 minute topographic quadrangle, Stahlstown, Pennsylvania.
- c. Size Classification - The height of the dam is 29 feet. The reservoir volume to the top of dam, elevation 1491.8 ft. M.S.L., is 36 acre-feet. Therefore, the dam is in the "Small" size category.
- d. Hazard Classification - There are a low number of habitable structures which might be affected by a failure of Breskin Pond No. 1. There is one residential structure located immediately downstream from the dam in which there is likely to be economic damage and loss of life if the dam were to fail. This house is approximately 5 to 10 feet above the streambed.

Breskin Pond No. 2 (NDI No. PA 00485, PennDER No. 65134) is located approximately 1700 feet downstream from Breskin Pond No. 1. Breskin Pond Dam No. 2 has been classified as a "High" hazard dam. One house and one summer cottage are located 2000 feet downstream of Breskin Pond No. 2. Economic damage to these structures, as well as to a secondary road and driveways, is likely to occur. However, failure of Breskin Pond No. 1 is not likely to cause loss of life in the damage center downstream from Breskin Pond No. 2. Breskin Pond No. 1 is therefore classified as a "Significant" hazard dam.

- e. Ownership - The dam is owned by Joseph Breskin, 506 MaGee Building, 336 Fourth Avenue, Pittsburgh, PA 15222.

f. Purpose of Dam - The reservoir created by the dam is used for recreation.

g. Design and Construction History - Breskin Pond Dam No. 1 was constructed in 1971 by Latimer Construction Company of New Alexandria, Pennsylvania. J. Fred Triggs, P.E., of Pittsburgh, Pennsylvania, and Ronald E. Kelley, P.E., of Greensburg, Pennsylvania, conducted separate engineering studies and made recommendations for modifications of the dam in 1972. Modifications of the dam in accordance with the plans and specifications prepared by Ronald E. Kelley were completed in 1973.

h. Normal Operating Procedures - The normal depth of the reservoir is approximately 4 feet (elevation 1472.6 ft. M.S.L.). The owner indicates that the impoundment has a leaky reservoir which cannot maintain a full pool; therefore, he cannot raise the reservoir level to the crest of the principal spillway.

1.3 PERTINENT DATA

a.	<u>Drainage Area (square miles)</u> -	0.2
b.	<u>Discharge at Dam Site (c.f.s.)</u> -	
	Maximum Flood	Unknown
	Spillway Capacity at Maximum Pool (El. 1491.8 ft. M.S.L.) -	154
c.	<u>Elevation* (feet above Mean Sea Level [ft. M.S.L.])</u> -	
	Design Top of Dam -	Unknown
	Minimum Top of Dam -	1491.8
	Maximum Design Pool -	Unknown
	Principal Spillway Crest -	1488.5
	Streambed at Toe of Dam -	1463.0
	Maximum Tailwater of Record -	Unknown
d.	<u>Reservoir (feet)</u> -	
	Length of Maximum Pool (El. 1491.8 ft. M.S.L.) -	600
	Length of Normal Pool (El. 1488.5 ft. M.S.L.) -	400

*All elevations are referenced to assumed elevation for the centerline of the adjacent roadway, El. 1500.0 ft. M.S.L., as assumed from the USGS 7.5 minute topographic quadrangle, Stahlstown, Pennsylvania.

e. Storage (acre-feet) -

Top of Dam (El. 1491.8 ft. M.S.L.) - 36
Normal Pool (El. 1488.5 ft. M.S.L.) - 29

f. Reservoir Surface (acres) -

Top of Dam (El. 1491.8 ft. M.S.L.) - 2.37
Normal Pool (El. 1488.5 ft. M.S.L.) - 1.84

g. Dam -

h. Diversions and Regulating Tunnels - None

i. Principal Spillway -

Type - 30-inch BCCMP riser pipe connected to a
24-inch BCCMP outlet conduit.
Location - Right-center of embankment.
Crest Elevation (ft. M.S.L.) - 1488.5
Gates - None
Downstream Channel - Spillway discharges near
downstream toe of dam and
flows to the original stream
channel.

j. Emergency Spillway -

Type - Trapezoidal earth channel
 Location - Right abutment
 Bottom Width of Channel Perpendicular
 to Flow (feet) - 6
 Side Slopes 5H:1V
 Crest Elevation (ft. M.S.L.) - 1489.9
 Gates - None
 Downstream Channel - Spillway discharges past
 embankment and into the
 original stream channel.

k. Outlet Works - 8-inch BCCMP controlled by Armco slide gate, operated from crest of embankment.

SECTION 2 - ENGINEERING DATA

2.1 DESIGN

The information reviewed consisted of File 65-141 of the Pennsylvania Department of Environmental Resources (PennDER). This file contained the following information:

- 1) The contract between Joseph and Kathleen Breskin and Latimer Construction Company of New Alexandria, Pennsylvania for the construction of two dams, dated 10 September 1971.
- 2) Various correspondence between Joseph Breskin and the Department of Forest and Waters and the Department of Environmental Resources regarding the design and safety of the dam.
- 3) An engineering report on the condition of Breskin Pond Dam No. 1 by J. Fred Triggs, P.E., dated 27 February 1972.
- 4) Test results and boring logs on the soils at the dam site by Pittsburgh Testing Laboratory, dated 10 June 1972.
- 5) Design plans for a spillway by J. Fred Triggs, P.E., dated 10 June 1972.
- 6) Design plans for an emergency spillway plan and an engineering report on the dam by Ronald E. Kelley, P.E., dated 24 June 1972.
- 7) Final report on completion of modifications to the Breskin Dams by Ronald E. Kelley, P.E., dated 20 July 1973.
- 8) An inspection report on Breskin Pond Dam No. 2, conducted on 26 May 1978, stating that Breskin Pond Dam No. 1 was found to be dewatered and water was flowing through the 8-inch diameter outlet pipe.

2.2 CONSTRUCTION

Breskin Pond Dam No. 1 was constructed in 1971 by Latimer Construction Company of New Alexandria,

Pennsylvania. The spillway of the dam was modified in accordance with plans and specifications prepared by Ronald E. Kelley, P.E., of Greensburg, Pennsylvania. This work was completed in July of 1973.

2.3 OPERATION

Mr. Joseph Breskin, the owner, and his caretaker, Elmer Lenhart, are responsible for the maintenance and operation of the dam. Mr. Breskin visits the dam daily. Maintenance procedures and a general inspection of the dam are performed monthly. However, there is no operation and maintenance manual.

2.4 EVALUATION

- a. Availability - PennDER File No. 65-141 contains the design drawings, correspondence and memorandums for Breskin Pond Dam No. 1.
- b. Adequacy - The information available is generally adequate for a Phase I Inspection.
- c. Validity - Observations and measurements performed during the visual inspection indicated a few deviations from the design drawings of the emergency spillway of the dam. These are:
 - 1) There is no thin layer of concrete over the embankment at the emergency spillway.
 - 2) There is no riprap on the right downstream embankment below the discharge channel of the emergency spillway.
 - 3) The emergency spillway discharge channel is not lined with riprap.

SECTION 3 - VISUAL INSPECTION

3.1 FINDINGS

- a. General - The dam was found to be in fair overall condition at the time of inspection on 5 December 1980. No unusual weather conditions were experienced during the inspection. The pool level was approximately 16 feet below riser crest (normal pool) level. Noteworthy deficiencies observed during the visual inspection are described briefly in the following paragraphs. The complete visual inspection checklist, field sketch, top of dam profile, and typical cross-section are given in Appendix A.
- b. Dam - The downstream slope is covered with brush and small saplings. A rodent hole was observed at Station 3+25, approximately 7 feet below the crest. The owner reports that the reservoir will not hold water. He reports that a rise of 3 to 4 inches occurs in his springhouse downstream from the dam when the lake is full. No outlet for seepage from the reservoir was observed in the immediate area of the dam during the inspection.
- c. Appurtenant Structures - An 8-inch Armco slide gate, which can be operated from near the crest on the upstream slope, serves as closure for the outlet pipe. The crank rods for this slide gate are bent; however, the owner reports the slide gate is operational. The discharge end of the principal spillway is located 6.5 feet above the downstream toe of the embankment. Because of the present configuration of the outlet end of the principal spillway, there is potential for scour and erosion of a portion of the embankment. At the time of the inspection, this area was not eroded; however, it should be examined in future inspections. The emergency spillway is located at the right abutment of the dam. Approximately half of the control section is excavated to bedrock; the other half consists of erodible soils without vegetation (i.e., grass) cover.
- d. Reservoir Area - The reservoir slopes are moderate with no signs of instability. Sedimentation is not a problem for this impoundment. The owner

indicates that this impoundment has a leaky reservoir which cannot maintain a full pool.

e. Downstream Channel - There is a two-story wooden frame residence approximately 200 feet downstream of the dam to the left of the downstream channel. Breskin Pond Dam No. 2 (NDI No. PA 00485, PennDER No. 65-134) is located approximately 1700 feet downstream of Breskin Pond Dam No. 1. Ackenheil and Associates has prepared a Phase I Inspection Report for Breskin Pond Dam No. 2, dated 21 August 1980. One house and one summer cottage are located 2000 feet downstream of Breskin Pond Dam No. 2. These may suffer economic damage if Breskin Pond Dam No. 1 were to fail.

SECTION 4 - OPERATIONAL PROCEDURES

4.1 PROCEDURES

There is a formal warning and evacuation plan in effect for Breskin Pond Dam No. 1. The plan was written in October 1980 and includes a plan to alert downstream occupants and local authorities of the procedures that should be followed in the event of a severe flood or imminent failure of the dam.

4.2 MAINTENANCE OF DAM

There are no formal records of maintenance or formal procedures for evaluating the necessity of maintenance for the structure. It is recommended that formal inspection procedures be developed.

4.3 MAINTENANCE OF OPERATING FACILITIES

The only operating facility at Breskin Pond Dam No. 1 is an 8-inch gate valve on the bottom of the upstream embankment. This gate valve is operated by a control at the upstream crest of the embankment. According to the owner, this valve and operating equipment is oiled and checked regularly by his caretaker. This valve is used to draw down the reservoir during the winter months.

4.4 DESCRIPTION OF ANY WARNING SYSTEM

There is a formal warning and evacuation plan in effect for Breskin Pond Dam No. 1. The plan was written in October 1980 and includes a plan to alert downstream occupants and local authorities of the procedures that should be followed in the event of a severe flood or imminent failure of the dam.

4.5 EVALUATION OF OPERATIONAL ADEQUACY

The current operational features are adequate for the purpose they serve. However, it is recommended that a formal maintenance and operations manual be prepared for this dam.

SECTION 5 - HYDRAULIC/HYDROLOGIC

5.1 EVALUATION OF FEATURES

- a. Design Data - No hydrologic or hydraulic design calculations are available for Breskin Pond Dam No. 1.
- b. Experience Data - There is no information available on the maximum reservoir level or discharge.
- c. Visual Observations - There is one low spot on the crest of the dam, located near the center of the embankment at Station 2+50. This spot is approximately 1.9 feet above the spillway crest.

The velocity in the emergency spillway during the 1/2 PMF is approximately 7 f.p.s. which will cause erosion of the unprotected channel. The spillway channel should have riprap or some other type of erosion protection.

No other problems were observed during the inspection which would indicate that the dam and appurtenant structures could not perform satisfactorily during a flood event.

- d. Overtopping Potential - Breskin Pond Dam No. 1 is a "Small" size - "Significant" hazard dam requiring evaluation for a spillway design flood (SDF) in the range of the 100-year flood to the 1/2 Probable Maximum Flood (1/2 PMF). Because of the small size of the impoundment, the 100-year flood was selected as the SDF.

The hydraulic capacity of the dam, reservoir and spillway was assessed by using the U.S. Army Corps of Engineers' Flood Hydrograph Package, HEC-1 DB. The hydrologic characteristics of the drainage basin, specifically the Snyder's unit hydrograph parameters, were obtained from a regionalized analysis conducted by the Baltimore District of the U.S. Army Corps of Engineers.

Analysis of the dam and spillway shows that the dam would be overtopped by the 100-year flood for a total duration of 0.60 hour by a maximum depth of 0.34 foot.

- e. Spillway Adequacy - As outlined in the above analyses, the spillway will not pass the SDF without overtopping the dam; therefore, the spillway is considered "Inadequate."

SECTION 6 - STRUCTURAL STABILITY

6.1 EVALUATION OF STRUCTURAL STABILITY

- a. Visual Observations - There were no structural inadequacies noted during the visual inspection that cause concern for the structural stability of the dam.
- b. Design and Construction Data - A boring was performed by Pittsburgh Testing Laboratories on 3 January 1972. This boring went through the embankment and into the underlying foundation. A shelby tube sample was extracted from a depth of 5.0 ft. to 6.5 ft. A sample from this shelby tube was sheared in a direct shear apparatus with the following results:

Cohesion (c) = 0.26 TSF
Angle of Internal Friction (ϕ) = 34.4°

A 27 February 1972 Engineering Report by J. Fred Triggs, P.E., contained the following information concerning the stability analysis:

"Stability analyses have been made in accordance with recommended procedures in 'Engineering for Dams,' by Justin, Hinds and Craeger, John Wiley and Sons, Inc., June 1957. These analyses show a Factor of Safety in Resistance to Sliding of $73914/24,500$ or 3.02; a Factor of Safety of Stability against Headwater Pressure of $51.53/14.84$ or 3.47; and a Factor of Stability in the case of Instant Drawdown of $42,058/20980$ or 2.004."

Since no sign of distress was observed in the field and the slopes have had a history of satisfactory performance, further assessments of the structural stability are not considered necessary. However, should future inspections observe signs of distress or seepage which would affect the structural stability of the dam, additional evaluations and corrective measures may become necessary.

- c. Operating Records - No operating records are available. Nothing in the procedures described by the owner indicates concern for the structural stability of the dam.

- d. Post-Construction Changes - No changes adversely affecting the structural stability of the dam have been performed.
- e. Seismic Stability - The dam is located in Seismic Zone 1 of the "Seismic Zone Map of the Contiguous United States," Figure 1, page D-30, "Recommended Guidelines for Safety Inspection of Dams." This is a zone of minor seismic activity; therefore, further consideration of the seismic stability is not warranted.

SECTION 7 - ASSESSMENT, RECOMMENDATIONS, REMEDIAL MEASURES

7.1 DAM ASSESSMENT

- a. Safety - Breskin Pond Dam No. 1 was found to be in fair overall condition at the time of inspection. The dam is a "Significant" hazard - "Small" size dam requiring a spillway capacity in the range of the 100-year flood to the 1/2 PMF. The 100-year flood was chosen as the SDF because the dam is on the low end of the "Small" size category in terms of storage capacity. As presented in Section 5, the spillway and reservoir are not capable of passing the 100-year flood without overtopping of the dam. During the 100-year flood, the depth and duration of overtopping are 0.34 foot and 0.60 hours, respectively. Therefore, spillway is considered to be "Inadequate".
- b. Adequacy of Information - The information available and the observations made during the visual inspection are considered sufficient for this Phase I Inspection Report.
- c. Urgency - The owner should immediately initiate the further investigation discussed in paragraph 7.1.d.
- d. Necessity for Additional Data/Evaluation - The hydraulic/hydrologic analysis performed in connection with this Phase I Inspection Report has indicated the need for additional spillway capacity. It is recommended that the owner of Breskin Pond Dam No. 1, under the guidance of a professional engineer, develop remedial measures to ensure that the dam will not be overtopped by the 100-year flood.

7.2 RECOMMENDATIONS/REMEDIAL MEASURES

The inspection and review of information revealed certain items of work which should be performed without delay by the owner. Item 1 below should be completed under the direction of a qualified professional engineer experienced in the design and construction of earth dams.

- 1) Develop remedial measures to ensure that the dam will not be overtopped by the 100-year flood.

- 2) Provide riprap or some means of protecting the emergency spillway channel from erosion during the SDF.
- 3) Cut the brush and saplings on the downstream slope.
- 4) Fill the rodent hole on the downstream face of the embankment.

A formal emergency warning system has been prepared. It is recommended that formal emergency operation procedures for the dam be prepared. It is further recommended that formal inspection, maintenance, and operation procedures and records be developed and implemented. These should be included in a formal maintenance and operations manual for the dam.

APPENDIX A

VISUAL INSPECTION CHECK LIST, FIELD SKETCH,
TOP OF DAM PROFILE, AND TYPICAL CROSS-SECTION

Check List
Visual Inspection
Phase 1

Name of Dam Breskin Pond Dam No. 1 County Westmoreland State PA Coordinates Lat. N40° 13.9'
NDI # PA 01141 PennDER # 65-141

Date of Inspection 5 December 1980 Weather Cloudy Temperature 35° F.

Pool Elevation at Time of Inspection 1472.57 ft. M.S.L.* Tailwater at Time of Inspection 1463.19 ft. M.S.L.

*All elevations referenced to assumed elevation for the centerline of the adjacent roadway,
El. 1500.0 ft. M.S.L. as assumed from USGS 7.5 minute topographic quadrangle, Stahlstown,
Pennsylvania.

Inspection Personnel:

Michael Baker, Jr., Inc.:

James G. Ulinski
Jeffrey S. Maze
Gary W. Todd

Owner's Representatives:

Joseph Breskin

James G. Ulinski Recorder

CONCRETE/MASONRY DAMS - Not Applicable

Name of Dam: BRESKIN POND DAM NO. 1

NDI # PA 01141

VISUAL EXAMINATION OF	OBSERVATIONS	REMARKS OR RECOMMENDATIONS
-----------------------	--------------	----------------------------

LEAKAGE

STRUCTURE TO ABUTMENT/EMBANKMENT JUNCTIONS	
--	--

DRAINS	
--------	--

WATER PASSAGES	
----------------	--

FOUNDATION	
------------	--

CONCRETE/MASONRY DAMS - Not Applicable

Name of Dam: BRESKIN POND DAM NO. 1
NDT # PA 01141

卷之三

REMARKS OR RECOMMENDATIONS

卷之三

SURFACE CRACKS CONCRETE SURFACES

STRUCTURAL CRACKING

VERTICAL AND HORIZONTAL ALIGNMENT

MONOGRAMS

CONSTRUCTION JOINTS

A-4

EMBANKMENT

Name of Dam	BRESKIN POND DAM NO. 1	NDI #	PA 01141	VISUAL EXAMINATION OF	OBSERVATIONS	REMARKS OR RECOMMENDATIONS
SURFACE CRACKS				UNUSUAL MOVEMENT OR CRACKING AT OR BEYOND THE TOE	None observed	
				SLOUGHING OR EROSION OF EMBANKMENT AND ABUTMENT SLOPES	None observed	

EMBANKMENT

Name of Dam BRESKIN POND DAM NO. 1

NDI # PA 01141

VISUAL EXAMINATION OFOBSERVATIONS

VERTICAL AND HORIZONTAL
ALIGNMENT OF THE CREST

No problem observed

RIPRAP FAILURES

None observed

VEGETATION

The downstream slope is covered
with brush and small saplings.

Cut the brush and saplings.

ANIMAL/RODENT HOLES

A rodent hole was observed at
approximate station 3+25, approx-
imately 7 ft. below the crest.

Fill the rodent hole.

EMBANKMENT

Name of Dam BRESKIN POND DAM NO. 1

NDI # PA 01141

<u>VISUAL EXAMINATION OF</u>	<u>OBSERVATIONS</u>	<u>REMARKS OR RECOMMENDATIONS</u>
<u>JUNCTION OF EMBANKMENT AND ABUTMENT, SPILLWAY AND DAM</u>	No problems observed	
<u>ANY NOTICEABLE SEEPAGE</u>	None observed. The owner reports that the reservoir will not hold water. He reported that a rise of 3 to 4 in. occurs in his springhouse downstream when the lake is full. No outlet for seepage from the reservoir was observed in the immediate area of the dam during the inspection.	
<u>STAFF GAGE AND RECORDER</u>	None	
<u>DRAINS</u>	None	

Name of Dam: BRESKIN POND DAM NO. 1
NDI # PA 01141

OUTLET WORKS

VISUAL EXAMINATION OF OBSERVATIONS REMARKS OR RECOMMENDATIONS

CRACKING AND SPALLING OF CONCRETE SURFACES IN OUTLET CONDUIT The outlet conduit is an 8-in. BCCMP.

INTAKE STRUCTURE

Submerged - not observable

OUTLET STRUCTURE

No problem observed

OUTLET CHANNEL

No problem observed

EMERGENCY GATE

An 8-in. Armco slide gate which can be operated from the upstream slope serves as upstream closure. The crank rods along the upstream slope are bent and distorted; however, the owner reports the slide gate is operational.

PRINCIPAL SPILLWAY	
Name of Dam:	BRESKIN POND DAM NO. 1
NDI #	PA 01141
VISUAL EXAMINATION OF	OBSERVATIONS
CONCRETE WEIR	The riser unit is a 30-in. BCCMP. No problem was observed.
APPROACH CHANNEL	The crest of the riser is protected with a trash rack.
DISCHARGE CHANNEL	A 24-in. BCCMP serves as an outlet for the principal spillway. The discharge invert of the pipe is 6.5 ft. above the toe of the dam. Some minor erosion has occurred, but nothing requiring remedial work at this time.
BRIDGE AND PIERS	None

EMERGENCY SPILLWAY

Name of Dam: BRESKIN POND DAM NO. 1

NDI # PA 01141

VISUAL EXAMINATION OF OBSERVATIONS REMARKS OR RECOMMENDATIONS

CONTROL SECTION

The emergency spillway is approximately half founded on resistant rock at the right abutment. The other half is erodible soils.

APPROACH CHANNEL

The approach channel is formed by a bench along the right abutment. No problem was observed.

DISCHARGE CHANNEL

The discharge channel is a grass-lined channel running along the right downstream abutment.

BRIDGE AND PIERS

None

INSTRUMENTATION			
<u>VISUAL EXAMINATION</u>	<u>OBSERVATIONS</u>	<u>REMARKS OR RECOMMENDATIONS</u>	
MONUMENTATION/SURVEYS	None		
OBSERVATION WELLS	None		
WEIRS	None		
PIEZOMETERS	None		
OTHER			

RESERVOIR

Name of Dam: BRESKIN POND DAM NO. 1

NDI # PA 01141

VISUAL EXAMINATION OF OBSERVATIONS REMARKS OR RECOMMENDATIONS

SLOPES

The reservoir slopes are moderate (5°-15°) and forested.

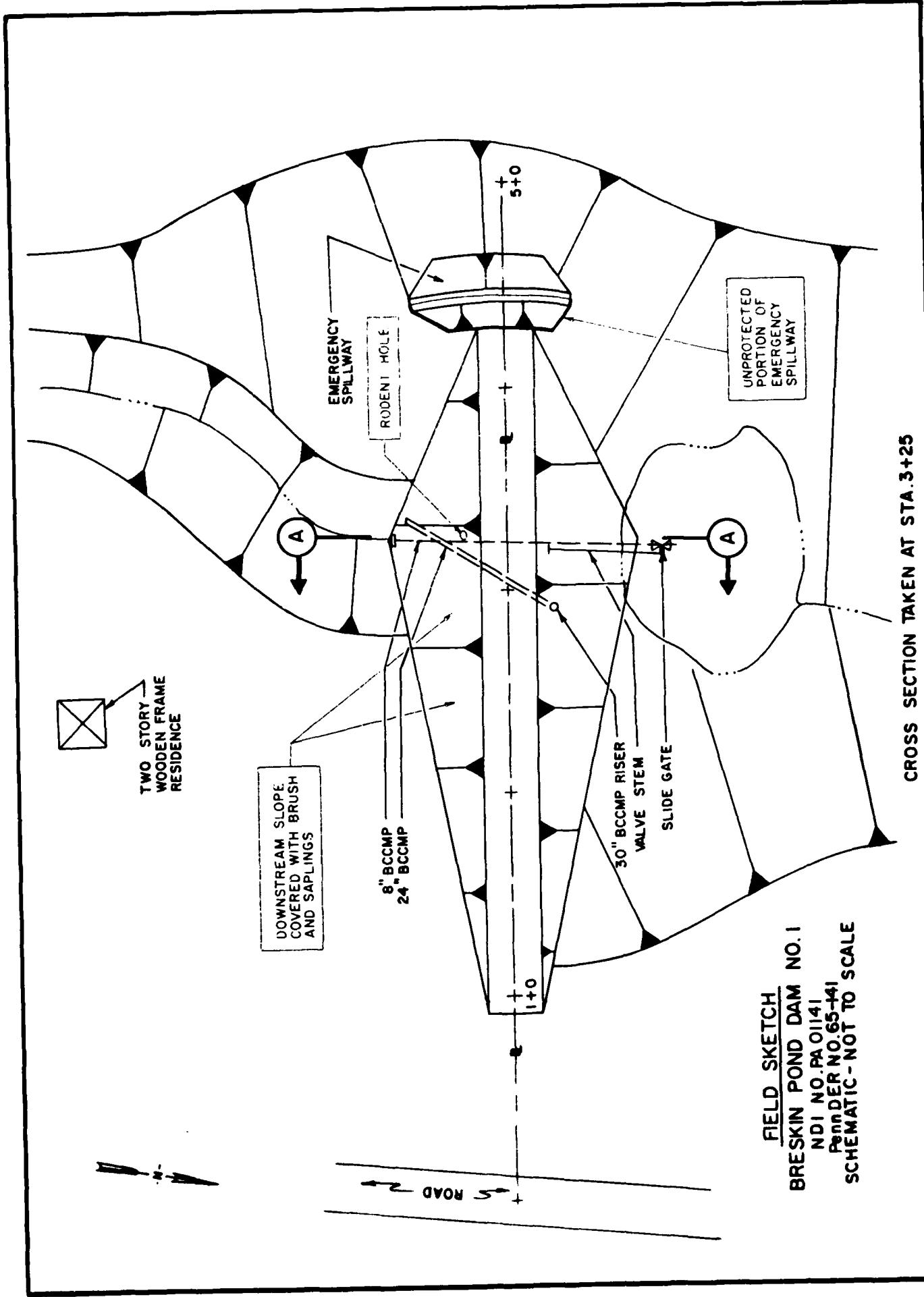
SEDIMENTATION

Sedimentation is not a problem for this impoundment.

DOWNSTREAM CHANNEL

Name of Dam: BRESKIN POND DAM NO. 1
NDI # PA 01141

VISUAL EXAMINATION OF CONDITION (OBSTRUCTIONS, DEBRIS, ETC.)	OBSERVATIONS	REMARKS OR RECOMMENDATIONS
SLOPES	The downstream channel slope is approximately 2% to the downstream pond.	
APPROXIMATE NO. OF HOMES AND POPULATION	There is a two story wooden frame residence, 5 to 10 ft. above the stream approximately 200 ft. downstream from the dam. Breskin Pond Dam No. 2 (NDI No. PA 00485, PENNDEP No. 65-134) is located approximately 1700 ft. downstream of Breskin Pond Dam No. 1. Ackenheil and Associates has prepared a Phase I Inspection Report for Breskin Pond Dam No. 2, dated 21 August 1980. There is 1 house and 1 summer cottage located 2000 ft. downstream of Breskin Pond Dam No. 2 which may suffer damage.	



FIELD SKETCH
BRESKIN POND DAM NO. 1
NDI NO. PA 01141
POND NO. 65-41
SCHEMATIC - NOT TO SCALE

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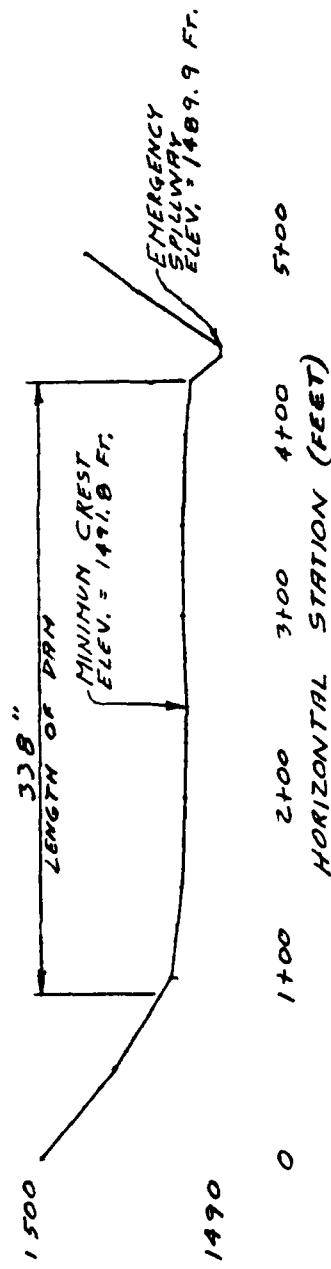
BRESKIN POND DAM NO. 1

TOP OF DAM PROFILE
TYPICAL CROSS-SECTION

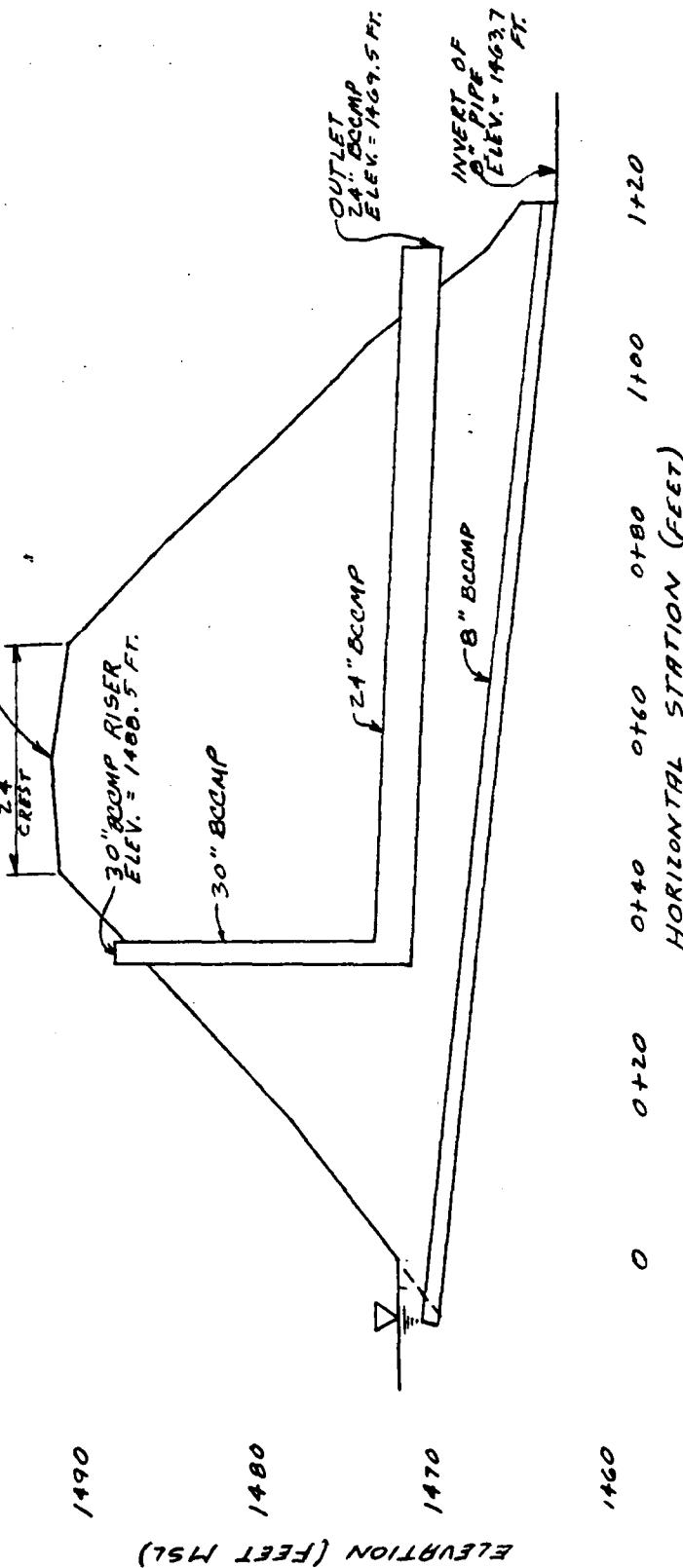
DATE OF INSPECTION: 5 December 1980

Top Of Dam Profile (looking downstream)

Length of Dam = 338 FEET



ELEVATION (FEET MSL)

Typical Cross Section At Sta. 3+25

ELEVATION (FEET MSL)

APPENDIX B
ENGINEERING DATA CHECK LIST

CHECK LIST
ENGINEERING DATA
DESIGN, CONSTRUCTION, OPERATION

Name of Dam: BRESKIN POND DAM NO. 1

NDI # PA 01141

ITEM **REMARKS**

PLAN OF DAM

See Plate 3 of this report.

REGIONAL VICINITY MAP

A USGS 7.5 minute topographic quadrangle, Stahlstown, Pennsylvania, was used to prepare the vicinity map which is enclosed in this report as the Location Plan (Plate 1).

CONSTRUCTION HISTORY

Breskin Pond Dam No. 1 was constructed by Latimer Construction Company of New Alexandria, Pennsylvania in 1971. Modifications of the dam in accordance with the plans and specifications prepared by Ronald E. Kelley were completed in 1973.

TYPICAL SECTIONS OF DAM

See Plate 4 of this report.

HYDROLOGIC/HYDRAULIC DATA

No information available

OUTLETS - PLAN

None available

- DETAILS

None available

- CONSTRAINTS

None

- DISCHARGE RATINGS

None

RAINFALL/RESERVOIR RECORDS

No records are kept.

Name of Dam: BRESKIN POND DAM NO. 1

NDI # PA 01141

ITEM REMARKS

DESIGN REPORTS

None available

GEOLOGY REPORTS

No geology reports are available for the dam. See Appendix F for the Regional Geology.

DESIGN COMPUTATIONS
HYDROLOGY & HYDRAULICS
DAM STABILITY
SEEPAGE STUDIES

A stability analysis and seepage determination was computed by J. Fred Triggs, P.E. A copy of the calculations, dated 27 February 1972, is available in the PennDER file.

MATERIALS INVESTIGATIONS
BORING RECORDS
LABORATORY
FIELD

A core boring was made at the center of the dam near the original streambed location to a depth of 43.8 ft. This core boring was made by Pittsburgh Testing Laboratory on 3 January 1972 and the results are available in the PennDER file. One direct shear test was performed on material from a Shelby tube sample (5.0 ft. to 6.5 ft. depth). The results were cohesion = 0.26 TSF and $\phi = 34.4^\circ$.

POST-CONSTRUCTION SURVEYS
OF DAM

No information available

BORROW SOURCES

The borrow source was obtained from the reservoir area and high on the right abutment.

Name of Dam: BRESKIN POND DAM NO. 1
NDI # PA 01141

B-3

ITEM	REMARKS
MONITORING SYSTEMS	None
MODIFICATIONS	The emergency spillway was reportedly modified in 1972 by paving it with a thin layer of concrete. However, there was no evidence of the pavement at the time of inspection.
HIGH POOL RECORDS	No records are kept.
POST-CONSTRUCTION ENGINEERING STUDIES AND REPORTS	None available
PRIOR ACCIDENTS OR FAILURE OF DAM DESCRIPTION REPORTS	None reported in the available information.
MAINTENANCE OPERATION RECORDS	No records are kept.

Name of Dam: BRESKIN POND DAM NO. 1
NDI # PA 01141

B-4

<u>ITEM</u>	<u>REMARKS</u>
SPILLWAY PLAN,	SECTIONS, and DETAILS
OPERATING EQUIPMENT PLANS & DETAILS	None available

CHECK LIST
HYDROLOGIC AND HYDRAULIC DATA
ENGINEERING DATA

DRAINAGE AREA CHARACTERISTICS: 0.2 sq. mi., moderate to steep
slopes, heavily wooded

ELEVATION TOP NORMAL POOL (STORAGE CAPACITY): 1488.5 ft. M.S.L.
(29 ac.-ft.)

ELEVATION TOP FLOOD CONTROL POOL (STORAGE CAPACITY): 1491.8 ft. M.S.L.
(36 ac.-ft.)

ELEVATION MAXIMUM DESIGN POOL: Unknown

ELEVATION TOP DAM: 1491.8 ft. M.S.L. (minimum top of dam elevation)

EMERGENCY SPILLWAY: _____

- a. Crest Elevation 1489.9 ft. M.S.L.
- b. Type Trapezoidal earth channel
- c. Width of Crest Parallel to Flow 80 ft. (along centerline)
- d. Length of Crest Perpendicular to Flow 6.0 ft. at bottom
- e. Location Spillover Right abutment
- f. Number and Type of Gates None

OUTLET WORKS: _____

- a. Type 30" BCCMP riser pipe with 24" BCCMP outlet pipe
- b. Location 125 ft. left of emergency spillway
- c. Entrance Inverts 1488.5 ft.
- d. Exit Inverts 1469.5 ft.
- e. Emergency Drawdown Facilities 8" BCCMP blow-off pipe

HYDROMETEOROLOGICAL GAGES: None

- a. Type _____
- b. Location _____
- c. Records _____

MAXIMUM NON-DAMAGING DISCHARGE Unknown

APPENDIX C

PHOTOGRAPH LOCATION PLAN AND PHOTOGRAPHS

DETAILED PHOTOGRAPH DESCRIPTIONS

Overall View - Overall View of Dam From Right Abutment

Photograph Location Plan

Photo 1 - View of Upstream Slope From Right Abutment

Photo 2 - View of Crest and Downstream Slope From Right Abutment

Photo 3 - View of Crest From Left Abutment

Photo 4 - View of Downstream Slope From Left Abutment

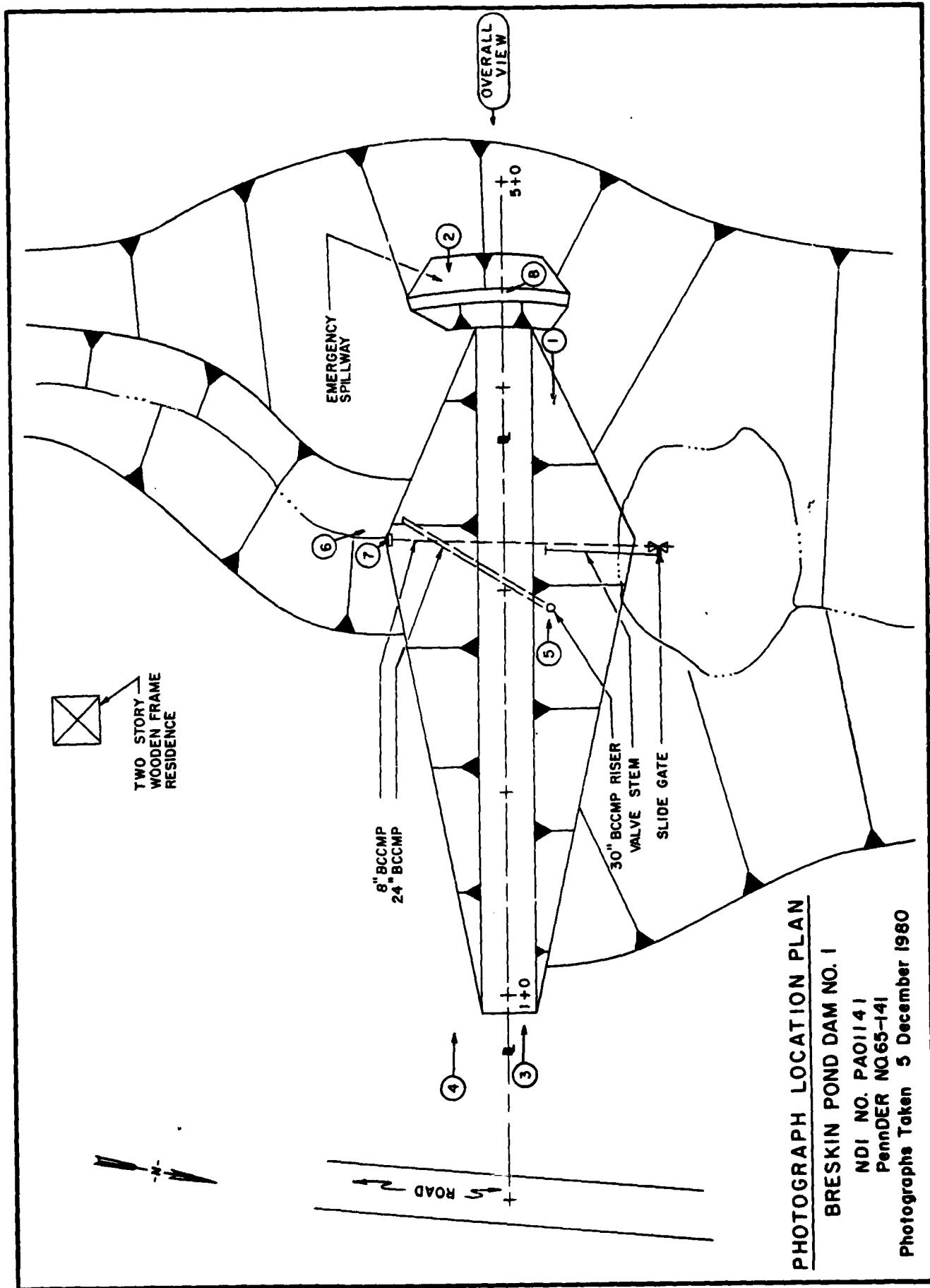
Photo 5 - View of Principal Spillway Riser Crest

Photo 6 - View of Downstream End of Principal Spillway and Outlet Works

Photo 7 - Close-up View of Downstream Outlet Works Headwall Structure

Photo 8 - View of Emergency Spillway Channel at Crest of Dam (Looking Downstream)

Note: Photographs were taken on 5 December 1980.



PHOTOGRAPH LOCATION PLAN

BRESKIN POND DAM NO. 1

NDI NO. PA01141

PennDER NQ65-141

Photographs Taken 5 December 1980

BRESKIN POND DAM No. 1

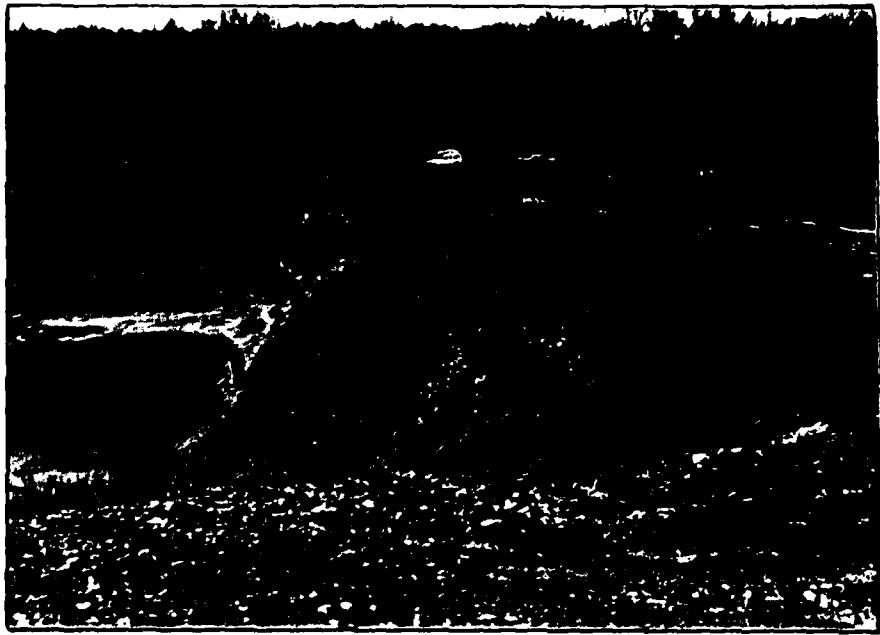


PHOTO 1. View of Upstream Slope from Right Abutment



PHOTO 2. View of Crest and Downstream Slope from Right Abutment

BRESKIN POND DAM No. 1



PHOTO 3. View of Crest from Left Abutment

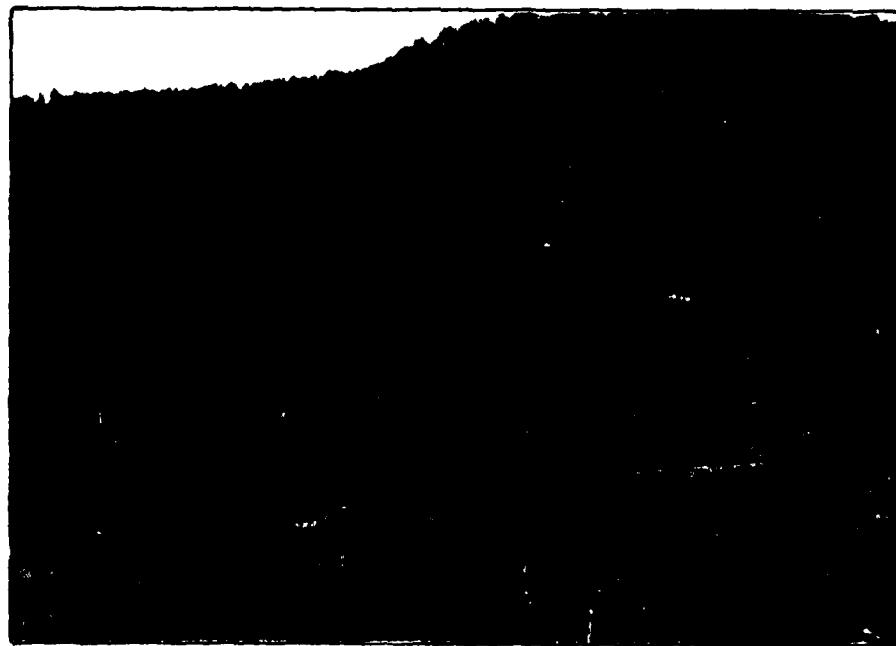


PHOTO 4. View of Downstream Slope from Left Abutment

BRESKIN POND DAM No. 1

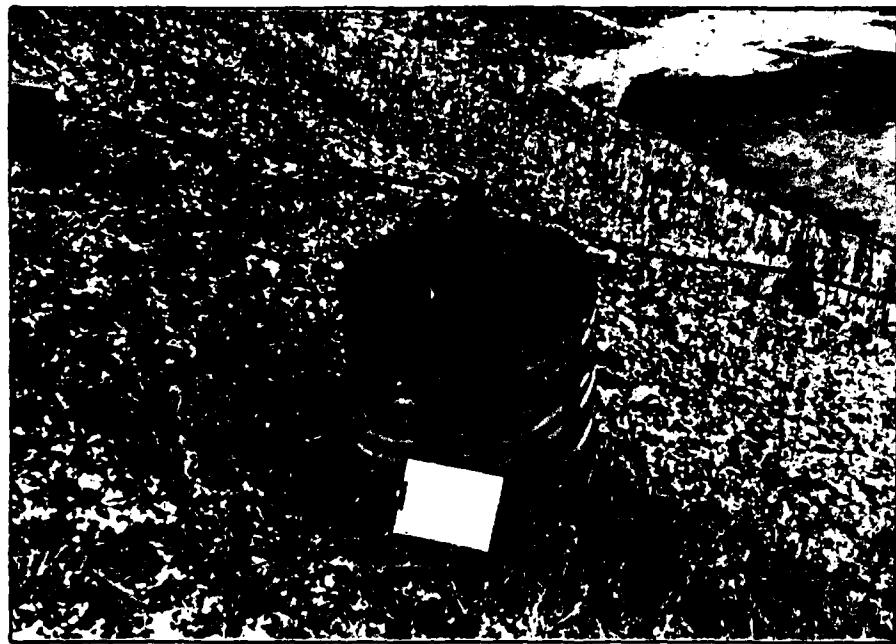


PHOTO 5. View of Principal Spillway Riser Crest

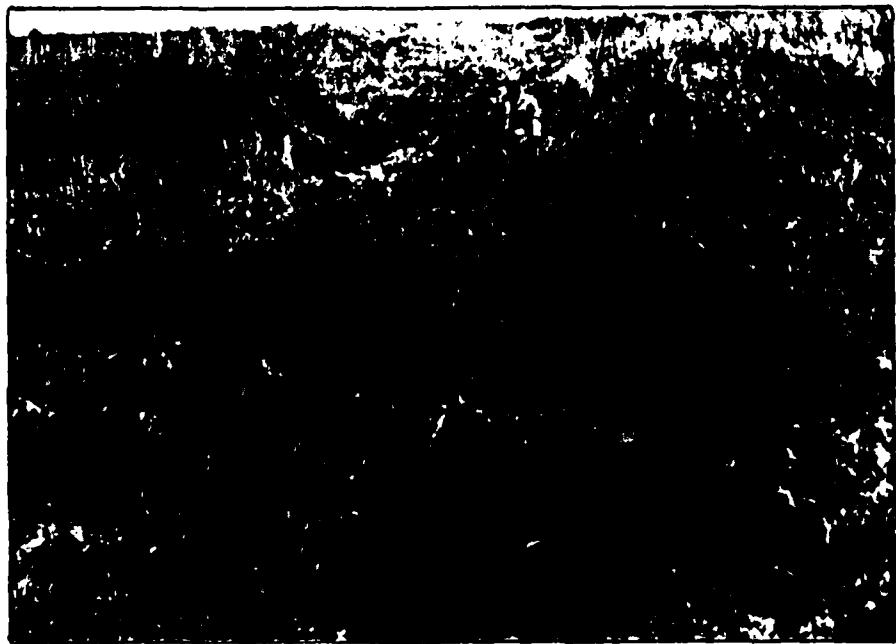
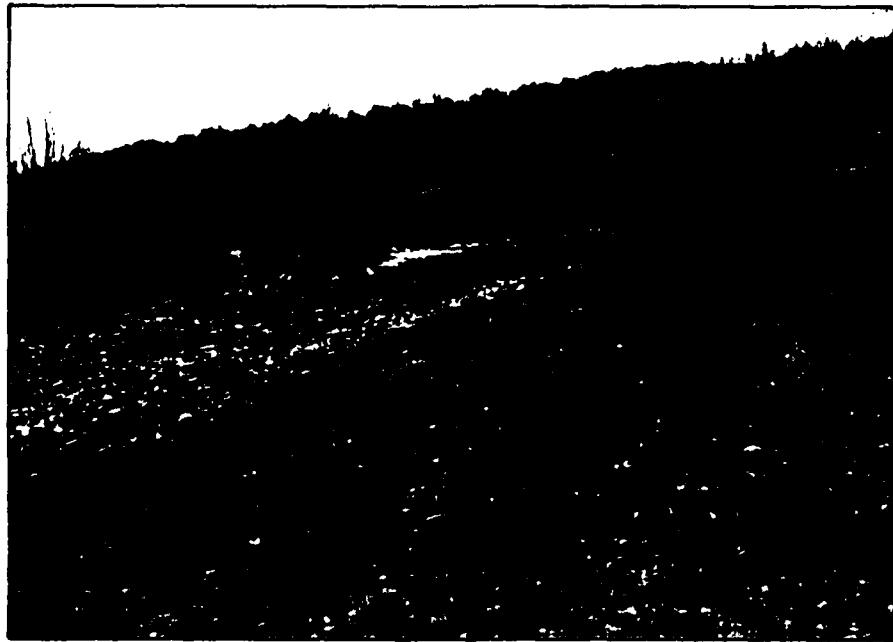


PHOTO 6. View of Downstream End of Principal Spillway and Outlet Works

BRESKIN POND DAM No. 1



PHOTO 7. Close-up View of Downstream Outlet Works Headwall Structure



**PHOTO 8. View of Emergency Spillway Channel at Crest of Dam
(Looking Downstream)**

APPENDIX D
HYDROLOGIC AND HYDRAULIC COMPUTATIONS

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THE BAKER ENGINEERS

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Beaver, Pa. 15009

Subject BRESKIN POND DAM No. 1 S.O. No. _____
APPENDIX D - HYDROLOGIC AND Sheet No. _____ of _____
HYDRAULIC CALCULATIONS Drawing No. _____
Computed by _____ Checked by _____ Date _____

TABLE OF CONTENTS

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EMERGENCY SPILLWAY RATING	9
COMBINED RATING SUMMARY	11
100-YEAR STORM DISTRIBUTION	12
100-YEAR FLOOD CALCULATION	13
HEC-1 COMPUTER ANALYSIS	15

PREFACE
HYDROLOGIC AND HYDRAULIC COMPUTATIONS

The hydrologic determinations presented in this Phase I Inspection Report are based on the use of a Snyder's unit hydrograph developed by the U.S. Army Corps of Engineers. Due to the limited number of gaging stations available in this hydrologic region and the wide variations of watershed slopes, the Snyder's coefficients may yield results of limited accuracy for this watershed. As directed however, a further refinement of these coefficients is beyond the scope of this Phase I Investigation.

In addition, the conclusions presented pertain to present conditions, and the effect of future development on the hydrology has not been considered.

HYDROLOGY AND HYDRAULIC ANALYSIS
DATA BASE

NAME OF DAM: BRESKIN POND DAM NO. 1

100- YEAR STORM = 5.5 INCHES/24 HOURS⁽¹⁾

STATION	1	2	3	4	5
Station Description	BRESKIN POND DAM No. 1				
Drainage Area (square miles)	0.2				
Cumulative Drainage Area (square miles)	0.2				
Adjustment of PMF for Drainage Area (%) ⁽¹⁾	100-YEAR STORM DISTRIBUTION ON SHEET 12				
6 Hours					
12 Hours					
24 Hours					
48 Hours					
72 Hours					
SCS Dimensionless Unit Hydrograph Parameters					
T_c = .94 Hr.					
Lag Time = 0.56 Hr.					
Spillway Data					
Crest Length (ft)					
Freeboard (ft)					
Discharge Coefficient					
Exponent					

(1) Technical Paper No. 40 Cooperative Studies Section, U.S. Weather Bureau, Washington, D.C., 1961.

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Subject BRESKIN DAM NO 1 S.O. No. _____
HYDRAULIC DATA Sheet No. 2 of 19
Computed by APK Checked by WDX Drawing No. _____
Date 1-5-81

STORAGE CALCULATIONS

AREA VS. ELEVATION (MEASURED FROM QUADS)

ELEVATION (FT)	SURFACE AREA (ACRES)
1488.5	1.84
1500.0	3.67
1520.0	5.51

NORMAL POOL STORAGE

$$\text{STORAGE VOLUME} = V_{NP} = \frac{h}{3} (A_1 + A_2 + \sqrt{A_1 A_2})$$

h = ESTIMATED AVERAGE DEPTH = 20.0 FT

A_1 = SURFACE AREA OF NORMAL POOL = 1.84 acre

A_2 = SURFACE AREA OF RESERVOIR BOTTOM = 1.10 acre
(ESTIMATED FROM AVERAGE DEPTH
AND RESERVOIR SIDE SLOPES)

$$\text{NORMAL POOL STORAGE} = V_{NP} = \frac{20}{3} (1.84 + 1.10 + \sqrt{1.84 \cdot 1.10})$$

$$V_{NP} = 29.1 \text{ acre-ft.}$$

TOP OF DAM STORAGE

36 acre-ft (FROM HEC-1 ANALYSIS)

SNYDERS UNIT HYDROGRAPH PARAMETERS

$$L = 0.83 \text{ mi} \quad L_{CA} = 0.30 \text{ mi}$$

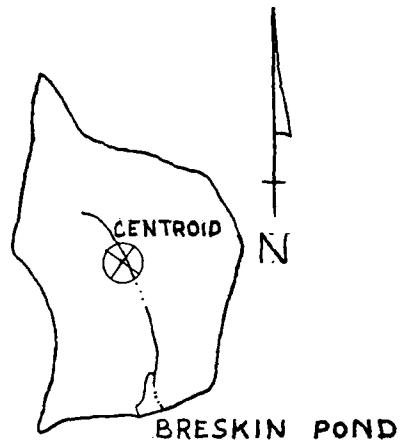
WATERSHED IS IN ZONE 24

$$C_T = 1.60 \quad C_p = 0.45$$

$$T_p = C_T (L \times L_{CA})^{0.3} = 1.6 (0.83 \times 0.30)^{0.3} = 1.06$$

DRAINAGE AREA = 0.2 SQUARE MILES

QUAD:
STAHLSTOWN



BRESKIN POND DAM:
DRAINAGE AREA AND
CENTROID MAP

0 2000 4000 6000



SCALE: 1" = 2000'

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Subject BRESKIN POND DAM No. 1

S.O. No. _____

TOP OF DAM PROFILE

Sheet No. 4 of 19

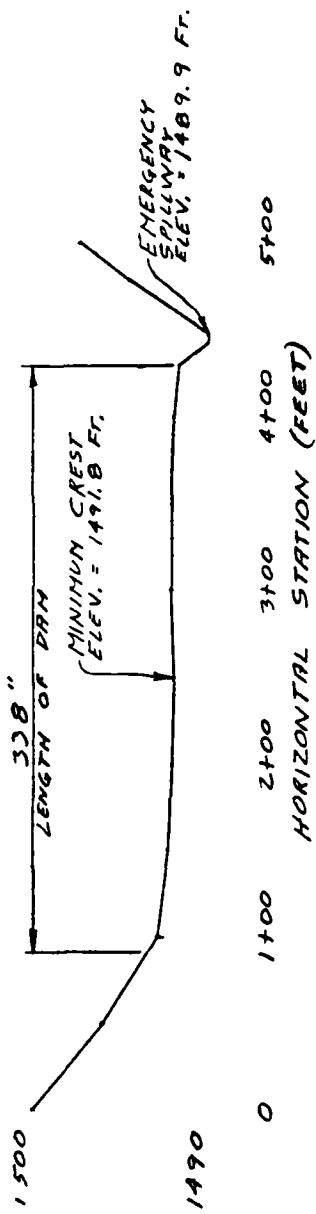
Typical Cross Section

Drawing No. _____

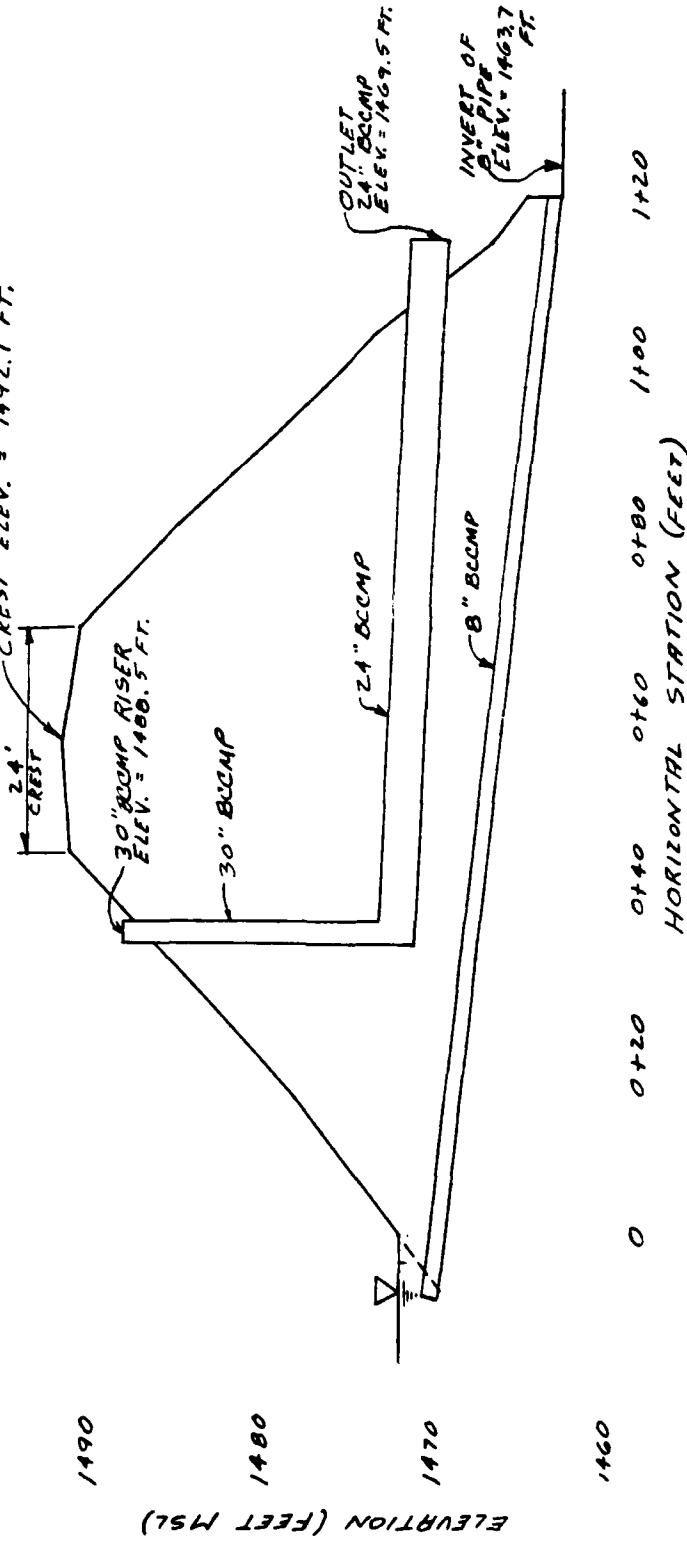
Computed by GWT Checked by WDL

Date 12-16-80

Top Of Dam Profile (looking downstream)
LENGTH OF DAM = 338 FEET



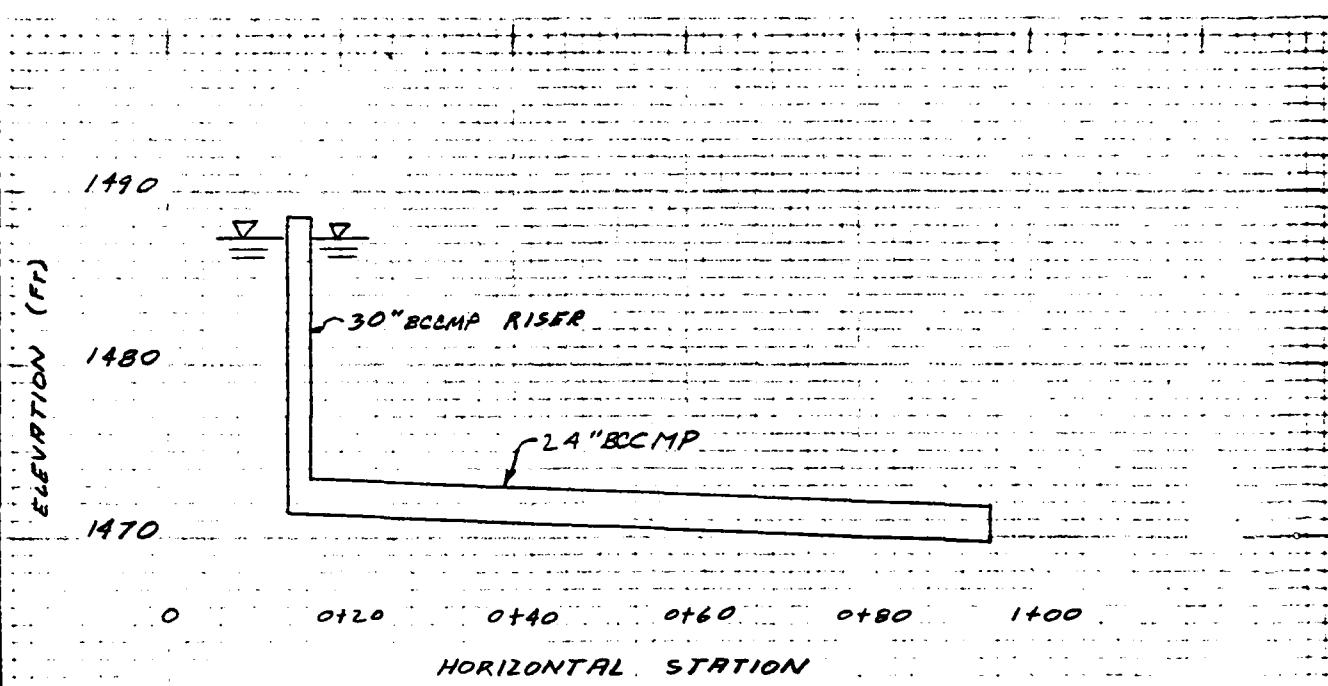
Typical Cross Section At Sta. 3+25



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Subject BRESKIN POND DAM No. 1 S.O. No. _____
OUTLET PIPE RATING Sheet No. 5 of 19
Computed by GWT Checked by WDL Drawing No. _____
Date 12/31/80



PIPE = 24" DIA. BCCMP

LENGTH = 86 FEET

TAILWATER = ELEV. 1463.7 FT.

RESERVOIR = ELEV. 1472.6 FT.

TOP OF RISER = ELEV. 1488.5 FT.

RISER TOP = 30" DIA. BCCMP

INLET FOR 24" DIA. PIPE = ELEV. 1471.5 FT.

OUTLET FOR 24" DIA. PIPE = ELEV. 1469.5 FT.

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Subject: BRESKIN POND DAM NO. 1

S.O. No. _____

OUTLET PIPE RATING

Sheet No. 6 of 19

Computed by GWT

Checked by WDL

Drawing No. _____
Date 12-31-60

WEIR FLOW - 30" DIA. INLET TOWER

$$Q = CLH^{3/2}$$

$$L \cdot C = 2\pi R \quad R = 1.25 \text{ FT.}$$

$$Q = 29.8 H^{3/2}$$

$$= 7.85 \text{ FT.}$$

H VARIES FROM 0 TO 6 FT.

C = 3.8 Pg. 5-41. BRATER & KING

ELEVATION, (FT)	C	(FT)	H (FT)	Q (CFS)
1488.5	3.8	7.85	0	0
1489.0	3.8	7.85	0.5	10.5
1489.5	3.8	7.85	1.0	29.8
1490.0	3.8	7.85	1.5	54.7
1490.5	3.8	7.85	2.0	84.3
1491.0	3.8	7.85	2.5	117.8
1491.5	3.8	7.85	3.0	154.8

ORIFICE FLOW - 30" DIA. INLET TOWER

$$Q = CA (2g H)^{0.5}$$

$$DIA. = 30" = 2.5 \text{ FT.}$$

$$= 0.6 (4.91) (2g H)^{0.5}$$

$$A = \pi R^2 = 4.91$$

$$= 23.64 (H)^{0.5}$$

$$g = 32.2 \text{ FT/SEC}^2$$

C = 0.6 Pg. 4-31. BRATER & KING.

H VARIES FROM 0 TO 6 FT.

ELEVATION, (FT)	H (FT)	Q (CFS)
1488.5	0	0
1489.0	0.5	16.7
1489.5	1.0	23.6
1490.0	1.5	28.9
1490.5	2.0	33.4
1491.0	2.5	37.4
1491.5	3.0	40.9
1492.0	3.5	44.2
1493.0	4.5	50.1
1494.0	5.5	55.4

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Subject BRESKIN POND DAM NO. 1 S.O. No. _____
OUTLET PIPE RATING Sheet No. 7 of 19
Computed by GWT Checked by WDL Drawing No. _____
Date 12/31/80

ORIFICE FLOW - 24" DIA. OUTLET PIPE

$$Q = CA (2gH)^{0.5}$$

$$Q = 0.6(3.14)(64.4H)^{0.5}$$

$$= 15.12 (H)^{0.5}$$

$$DIA = 24" = 2.0 \text{ FT}$$

$$A = \pi R^2 = 3.14 \text{ SQ. FT.}$$

$$g = 32.2 \text{ FT/SEC}^2$$

$$C = 0.6 \text{ Pg 4-31 + 4-32 BRATER + KING}$$

H VARIES FROM 16.0 FT. TO 22.0 FT.
AND IS MEASURED FROM THE CENTER
OF THE PIPE ELEV. 1472.5 FT.

ELEVATION, (FT)	H, (FT)	Q, (CFS)
1488.5	0	0
1489.0	16.5	61.4
1489.5	17.0	62.3
1490.0	17.5	63.2
1490.5	18.0	64.1
1491.0	18.5	65.0
1492.0	19.5	66.8
1493.0	20.5	68.4
1494.0	21.5	70.1

PIPE FLOW - 24" DIA. OUTLET PIPE

$$Q = \frac{A (2gH)^{1/2}}{D + K_d + K_s (L)}^{1/2}$$

$$= \frac{3.14 (64.4 H)^{1/2}}{1 + .78 + 0.0423 (80)}^{1/2}$$

$$Q = 10.82 H^{1/2}$$

$$A = 3.14 \text{ SQ. FT.}$$

$$g = 32.2 \text{ FT/SEC}^2$$

$$L = 86 \text{ FT.}$$

$$K_d (K_s) = 0.78 \text{ Pg 5.5-6 SCS NEH 5}$$

$$K_s (K_d) = 0 \text{ Pg 5.5-10 SCS NEH 5}$$

$$K_d (K_s) = 0.0423 \text{ Pg 5.5-4 SCS NEH 5}$$

$$n = 0.024$$

H VARIES FROM 17 FT. TO 23 FT. AND IS MEASURED FROM THE TOP OF PIPE ELEV. AT THE OUTLET ELEV 1471.5 FT.

ELEVATION, (FT)	H, (FT)	Q, (CFS)
1488.5	0	0
1489.0	17.5	45.3
1489.5	18.0	45.9
1490.0	18.5	46.5
1490.5	19.0	47.2
1491.0	19.5	47.8
1492.0	20.5	48.9
1493.0	21.5	50.2
1494.0	22.5	51.3

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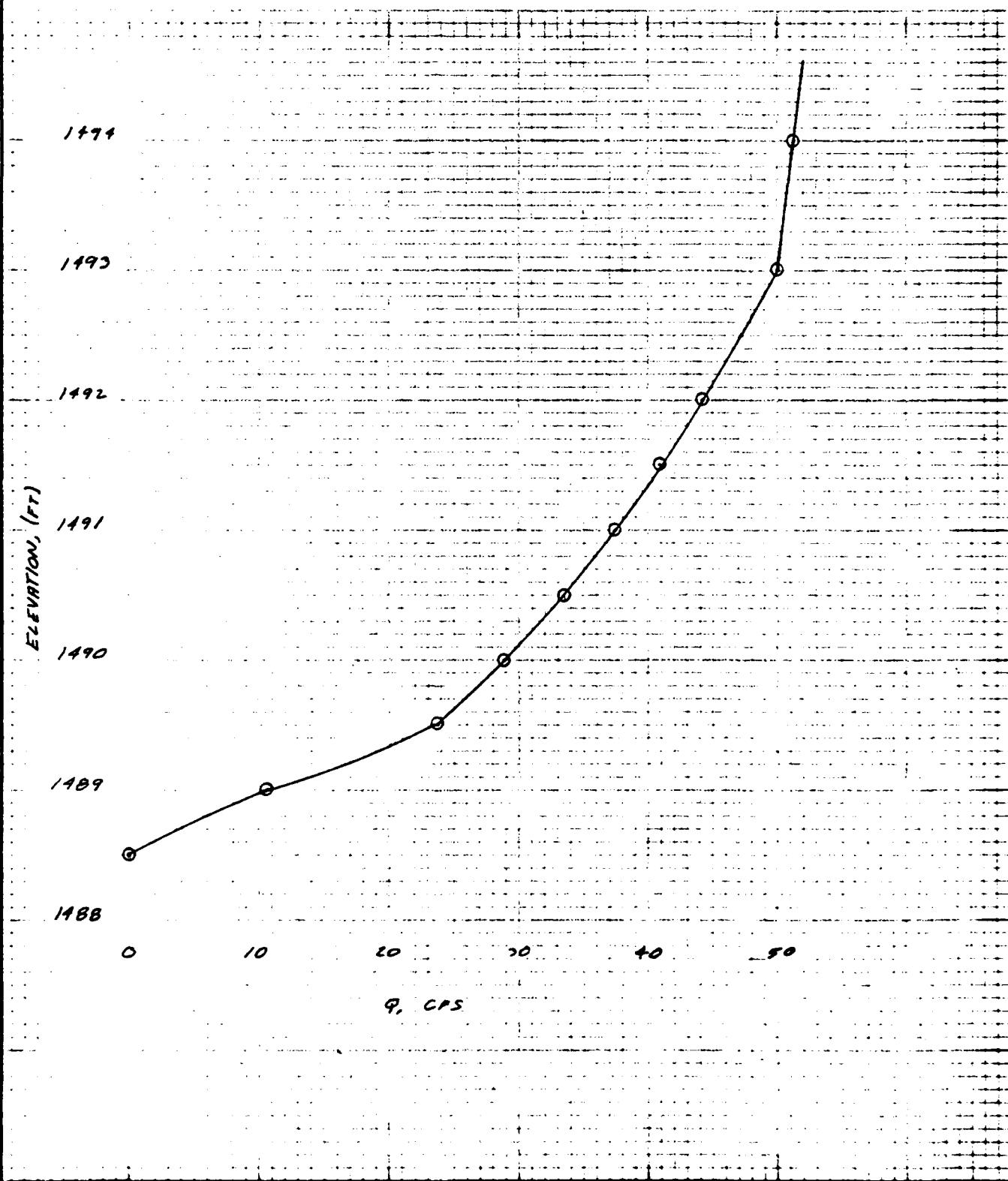
Box 280
Beaver, Pa. 15009

Subject BRESKIN POND DAM No. 1 S.O. No. _____

OUTLET PIPE RATING CURVE Sheet No. 8 of 19

Computed by GWT Checked by WDL Drawing No. _____

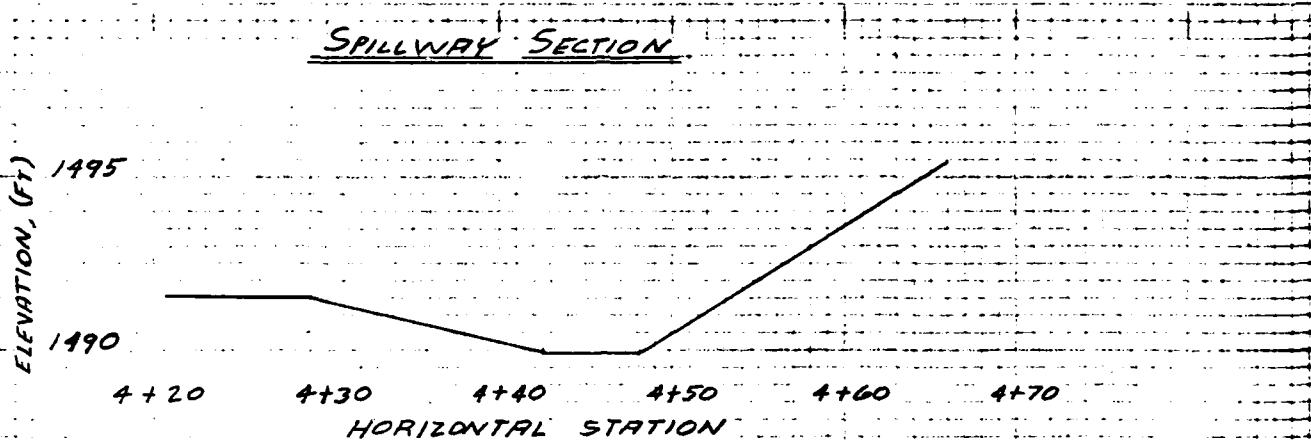
Date 1-2-81



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Subject BRESKIN POND DAM NO. 1 S.O. No. _____
EMERGENCY SPILLWAY RATING Sheet No. 9 of 19
Drawing No. _____
Computed by GWT Checked by WDL Date 1-2-81



DEVELOPED RATING CURVE BASED UPON CRITICAL FLOW OVER SPILLWAY:

$$V = \sqrt{gD} \quad (\text{CHOW, OPEN CHANNEL HYDRAULICS, p. 43})$$

$$g = 32.2 \text{ FT/SEC}^2$$

$$D = \text{MEAN HYDRAULIC DEPTH} = \frac{\text{FLOW AREA}}{\text{FREE SURFACE TOP WIDTH}} = \frac{A}{T}$$

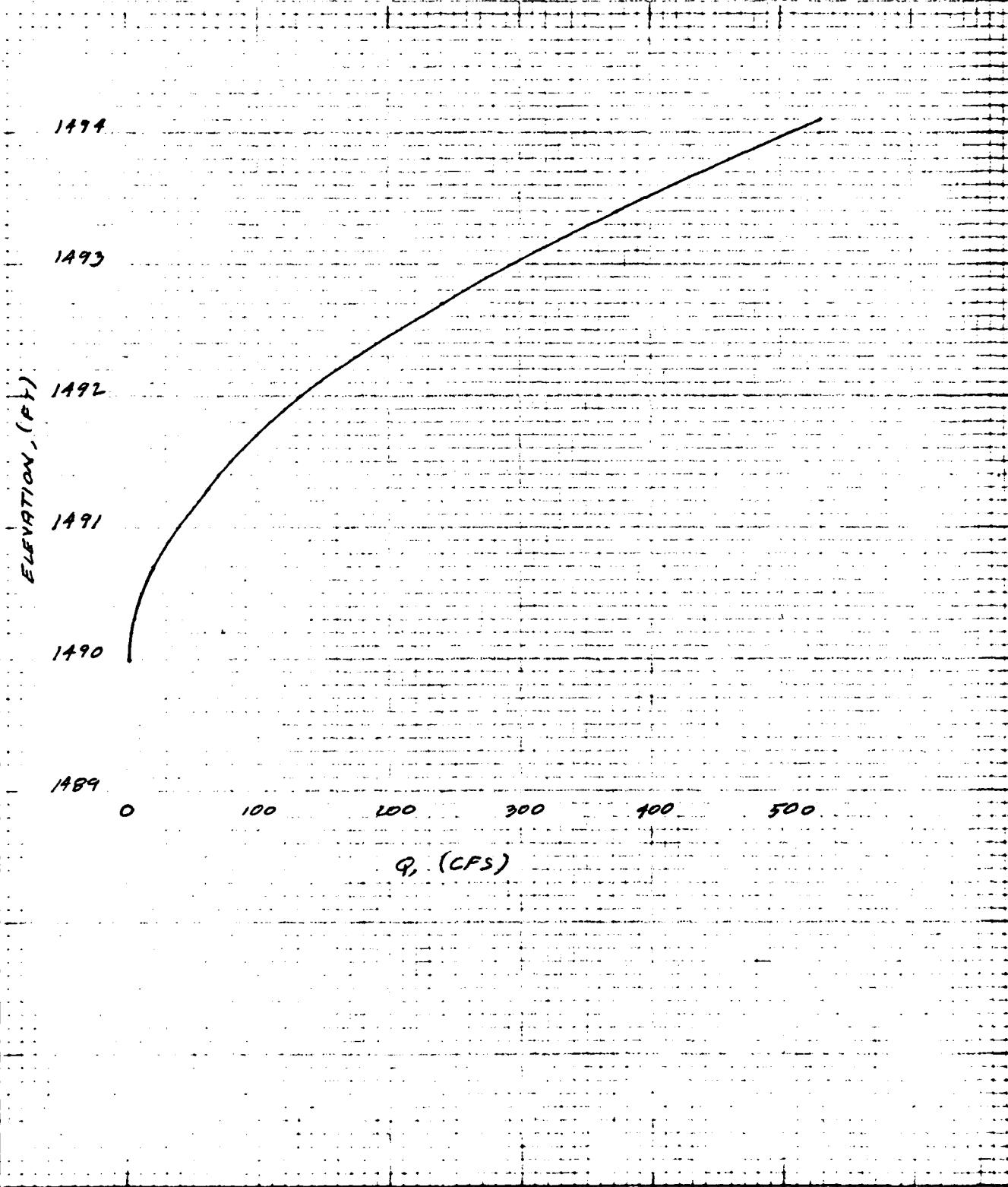
$$V = \text{MEAN FLOW VELOCITY}$$

$$Q = VA$$

ELEVATION (FT)	FLOW DEPTH (FT)	AREA (FT ²)	TOP WIDTH (FT)	A/T	V. (FT/SEC)	Q. (CFS)	V ² /2g	RESERVOIR ELEVATION (FT)
1489.9	0	0	5.5	0	0	0	0	1489.9
1490.0	0.1	0.6	6.5	0.09	1.70	1.02	0.04	1490.0
1490.5	0.6	5.4	12.5	0.13	3.72	20.09	0.21	1490.7
1491.0	1.1	13.2	18.5	0.71	4.78	63.10	0.35	1491.3
1491.5	1.6	23.6	24.0	0.98	5.62	132.63	0.49	1492.0
1492.0	2.1	36.1	26.0	1.39	6.69	241.51	0.69	1492.7
1492.5	2.6	49.3	27.5	1.79	7.59	374.19	0.89	1493.4
1493.0	3.1	63.3	29.0	2.18	8.38	530.45	1.09	1494.1
1493.5	3.6	78.1	30.5	2.56	9.08	709.15	1.28	1494.8
1494.0	4.1	93.6	32.0	2.93	9.71	908.86	1.46	1495.5

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Subject BREKIN Pond Dam No. 1 S.O. No. _____
EMERGENCY SPILLWAY RATING Sheet No. 10 of 19
CURVE Drawing No. _____
Computed by GCLT Checked by _____ Date 1-2-81



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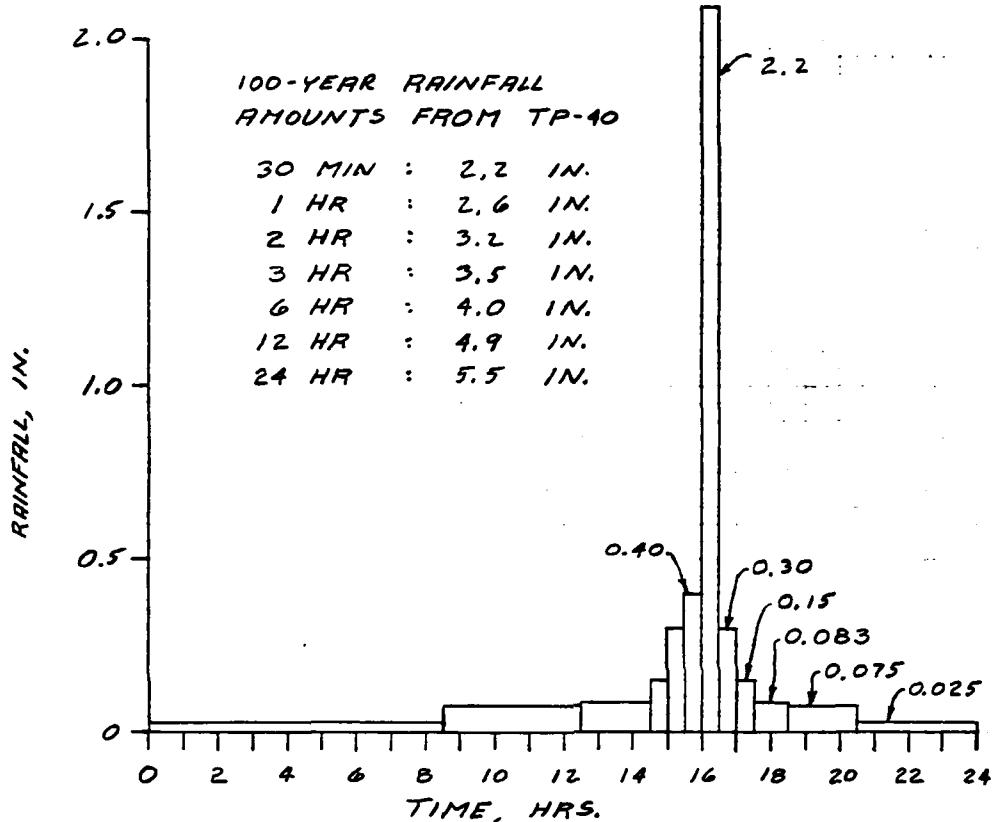
Subject BRESKIN POND DAM NO. 1 S.O. No. _____
SPILLWAY AND PIPE SUMMARY Sheet No. 11 of 19
Computed by GURT Checked by _____ Drawing No. _____
Date 1-2-81

EL E V A T I O N (F T)	OUTLET PIPE, Q. (CFS)	SPILLWAY, Q. (CFS)	TOTAL Q. (CFS)
1488.5	0	0	0
1489.0	10.5	0	10.5
1489.5	23.6	0	23.6
1490.0	28.9	1.0	29.9
1490.5	33.4	11.0	44.4
1491.0	37.4	40.0	77.4
1491.5	40.9	80.0	120.9
1492.0	44.2	132.6	176.8
1492.5	47.2	209.0	256.2
1493.0	50.1	295.0	345.1
1493.5	50.6	395.0	445.6
1494.0	51.3	530.0	581.3

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Subject BRESKIN POND DAM NO. 1 S.O. No. _____
100-YEAR STORM DISTRIBUTION Sheet No. 12 of 19
Computed by GWT Checked by WDL Drawing No. _____
Date 4/15/81



RAINFALL DISTRIBUTION
(30 MINUTE INTERVAL)

INTERVAL NUMBERS	% TOTAL RF OCCURRING IN EACH INTERVAL
1 - 17	0.5
18 - 25	1.3
26 - 29	1.5
30	2.7
31	5.4
32	7.2
33	40.0
34	5.4
35	2.7
36 - 37	1.5
38 - 41	1.3
42 - 48	0.5

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Subject BRESKIN POND DAM No. 1 S.O. No. _____
100-YEAR FLOOD CALCULATION Sheet No. 13 of 19
FROM OHIO RIVER REGRESSION EQUATION Drawing No. _____
Computed by GUVT Checked by WDY Date 4/15/81

CALCULATION OF 100-YEAR FLOW

FROM ANALYSIS PERFORMED BY THE
PITTSBURGH DISTRICT, CORPS OF ENGINEERS

$$Q_{100} = 120.38 (D.A. \times S^{1/2})^{0.744099}$$

D.A. = DRAINAGE AREA = 0.20 SQ. MI.

S = CHANNEL SLOPE IN THE LOWER 0.7 OF
THE WATERSHED = FT/MI.

$$S = \frac{1700 - 1488.5}{0.83} = 254.8$$

$$Q_{100} = 120.38 [(0.20)(254.8)^{1/2}]^{0.744099}$$

$$Q_{100} = 284 \text{ CFS}$$

USING ZERO LOSS RATES, A PEAK FLOW OF 193 CFS.
WAS OBTAINED IN THE HEC-1 ANALYSIS IF THE SNYDER'S
UNIT HYDROGRAPH PARAMETERS ORIGINALLY DERIVED FOR
THIS BASIN WERE USED.

THE 100-YEAR FLOOD HYDROGRAPH IS THEREFORE COMPUTED
USING THE SCS DIMENSIONLESS UNIT HYDROGRAPH APPROACH.
TIME OF CONCENTRATION AND LAG TIME ARE COMPUTED AS
FOLLOWS:

$$T_c = \text{TIME OF CONCENTRATION} = \text{OVERLAND FLOW TIME} + \text{CHANNEL FLOW TIME}$$

OVERLAND FLOW TIME

$$\text{DISTANCE} = 1850$$

$$\text{SLOPE} = \frac{1745 - 1625}{1850} = 6.5 \text{ %}$$

AVERAGE FLOW VELOCITY = 0.60 FT/SEC
(FROM FIG. 3.1, T.R. NO. 55 URBAN HYDROLOGY
FOR SMALL WATERSHEDS, SCS.)

$$\text{TRAVEL TIME} = 3.083 \text{ SEC.}$$

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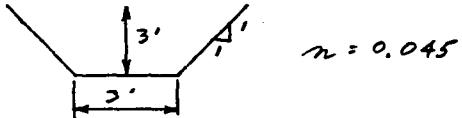
Subject BREISKIN Pond Dam No. 1 S.O. No. _____
100-YEAR Flood Calculation Sheet No. 14 of 19
CONTINUED Drawing No. _____
Computed by GWT Checked by WDX Date 4/15/81

CHANNEL FLOW TIME

DISTANCE = 2300 FT.

$$\text{SLOPE} = \frac{1625 - 1988.5}{2300} = 5.9 \%$$

ASSUME AVERAGE CHANNEL SIZE IS:



$$\text{AVERAGE FLOW VELOCITY} = V = \frac{1.49}{n} R^{2/3} S^{1/2}$$

$$V = \frac{1.49}{0.045} \left(\frac{(3+2)3}{3+6\sqrt{0.045}} \right)^{2/3} (0.059)^{1/2}$$

$$V = 7.04$$

$$\text{TRAVEL TIME} = 326 \text{ SEC.}$$

$$\text{TOTAL TRAVEL TIME} = T_c = 3083 + 326 = 3409 \text{ SEC.} \\ = 0.94 \text{ HR.}$$

$$\text{LAG TIME} = 0.6 T_c = .56 \text{ HR.}$$

WITH THE SCS PROCEDURE, A CURVE NUMBER OF 69 PRODUCED A PEAK FLOW OF 286 CFS. THIS VALUE IS WITHIN 1 % OF THE PREVIOUSLY COMPUTED PEAK FLOW OF 284 CFS AND IS WITHIN THE 10 % LIMIT SUGGESTED BY THE CORPS GUIDELINES.

***** FILE 10 HYDROGRAPH PACKAGE THEC-11
DATA SAFETY VERSION 11
LAST MODIFICATION 26 FEB 79
MNU UPDATE 04 JUN 79

FLUJ HYDROGRAPH PACKAGE (HEC-11)		DAY SAFETY VERSION		JULY 1978					
LAST MODIFICATION		26 FEB 79		JUN 79					
MBJ UPDATE									
NATIONAL PROGRAM FOR INSPECTION OF NON-FEDERAL DAMS									
HYDROLOGIC AND HYDRAULIC ANALYSIS OF BRESKIN POND DAM									
No. 1									
1	A1	K1		RUNOFF HYDROGRAPH TO DAM					
2	A2	K2		0.20					
3	A3	K3		5.5					
4	B	3.0	0	0.05	0.05				
5	B1	5	6	0.05	0.05				
6	J	1	1	0	0				
7	J1	1	1	0	0				
8	K	1	1	0	0				
9	M	3	2	0.05	0.05				
10	N	0	0.8	0.05	0.05				
11	O	0.05	0.05	0.05	0.05				
12	P	0.05	0.05	0.05	0.05				
13	Q	0.05	0.05	0.05	0.05				
14	R	0.05	0.05	0.05	0.05				
15	S	0.05	0.05	0.05	0.05				
16	T	0.05	0.05	0.05	0.05				
17	U	0.05	0.05	0.05	0.05				
18	V	0.05	0.05	0.05	0.05				
19	W	0.05	0.05	0.05	0.05				
20	X	0.05	0.05	0.05	0.05				
21	Y	0.05	0.05	0.05	0.05				
22	Z	0.05	0.05	0.05	0.05				
23	Y1	1	1	1	1				
24	Y2	1	1	1	1				
25	Y3	1	1	1	1				
26	Y4	1	1	1	1				
27	Y5	1	1	1	1				
28	SA	1.1	1.1	1.1	1.1				
29	SE	1.488.5	1.488.5	1.488.5	1.488.5				
30	SE	1.488.5	1.488.5	1.488.5	1.488.5				
31	SD	1.491.8	2.83	1.5	2.88				
32	SL	1.53	3.10	3.35	3.62				
33	SV	1.492.0	1.492.5	1.493.0	1.494.0				

SHEET 15 OF 19

FLOOD HYDROGRAPH PACKAGE (HEC-11)
DAN SAFETY VERSION JULY 1973
LAST MODIFICATION 26 Feb 79
M6.5 UPDATE 04 JUN 79

RUN DATE 04/16/81

WATERSHED PROGRAM FOR INSPECTION OF NIN-FEDERAL DAMS
HYDROLOGIC AND HYDRAULIC ANALYSIS OF BRESKIN POND DAM
WATERSHED HYDROGRAPH BY SCS METHOD

108 SPECIAL EDUCATION

NG	NR	NNVV	DAY	THUR	FRIDAY	MEIRC	TPUT	NSTAN
300	3	6	0	0	0	0	3	0
			JOPER	NWT	LROP1	TRACE	-4	0
			5	0	0	0	0	0

MULTI-PLAN ANALYSES TO BE PERFORMED

RT1125 = 1-30

SIB-ABEA BUNDESKOMMISSION

卷之三

卷之三

CURVE CN = -69.00 NETNESS = -1.00 EFFECT CN = 69.00

TC = 0.0 UNL HUKGRAPH DATA
LAG = 0.56

[1]

HYDROGRAPH ROUTING

RUTTLES FJR BRESKIN POND DAM NO. 1

ISTAG	ICOMP	IECON	ITAPE	JPLT	JPRI	INAE	ISTAGE	IAUTO
2	1	0	0	0	0	1	0	0
LOSS	CLOSS	Avg	ITRS	ISAME	IOP1	IPHP	LSIR	
0.0	0.0	0.0	1	1	0	0	0	
VSPTS	NSTOL	LAG	AMSKK	X	TSK	STORA	ISPRAY	
1	0	0	0.0	0.0	0.0	-1699.	-1	
STAGE	1488.50	1489.00	1489.50	1490.00	1490.50	1491.00	1491.50	1492.00
	1491.50	1494.00						1492.50
FLOW	0.0	10.50	23.60	29.90	44.40	77.40	120.90	176.80
	447.60	581.30						256.20
SURFACE AREA	1.	2.	4.	6.				
CAPACITY	0.	29.	60.	121.				
ELEVATION	1469.	1489.	1500.	1520.				
	1488.5	SPWID	CQW	EXPW	ELEV	CQ1L	CAREA	EXPL
		0.0	0.0	0.0	0.0	0.0	0.0	0.0
					DAY DATA			
					TOPEL	CQD	EXPD	JANWID
					1491.8	2.8	1.5	288.
CREST LENGTH	0.	150.	310.	335.	342.	350.		
AT OR BELOW ELEVATION	1491.8	1492.0	1492.5	1493.0	1493.5	1494.0		
PEAK OUTFLOW IS	255.	AT TIME	\$:00 HOURS					

SHEET 17 OF 19

PEAK FLOW AND STORAGE (END OF PERIOD) SUMMARY FOR MULTIPLE PLAN-RATIO ECONOMIC COMPUTATIONS
 FLOWS IN CUBIC FEET PER SECOND (CUBIC METERS PER SECOND)
 AREA IN SQUARE MILES (KILOMETERS)

OPERATION	STATION	AREA	RATIOS APPLIED TO FLOWS		1.00
			PLAN	RATIO 1	
HYDROGRAPH AT	1	0.22	1	286.	
ROUTED TO	2	0.22	1	255.	

SHEET 18 OF 19

SUMMARY OF DAM SAFETY ANALYSIS

PLAN 1	ELEVATION	INITIAL VALUE	SPILLWAY CREST	TOP OF DAM
STORAGE	29.	1488.50	1488.50	1491.80
OUTFLOW	0.		29.	36.

RATIO OF RESERVOIR PWF 4.5-ELEV	MAXIMUM DEPTH OVER DAM	MAXIMUM STORAGE AC-FT	MAXIMUM OUTFLOW CFS	DURATION HOURS	TIME OF OVER TOP HOURS	MAX OUTFLOW CFS	TIME OF FAILURE HOURS
1.00	1492.14	0.34	37.	255.	0.60	4.00	0.0

100 -YEAR FLOOD PREDICTION

BRESEN POND DAM NO. 1

SHEET 19 OF 19

APPENDIX E

PLATES

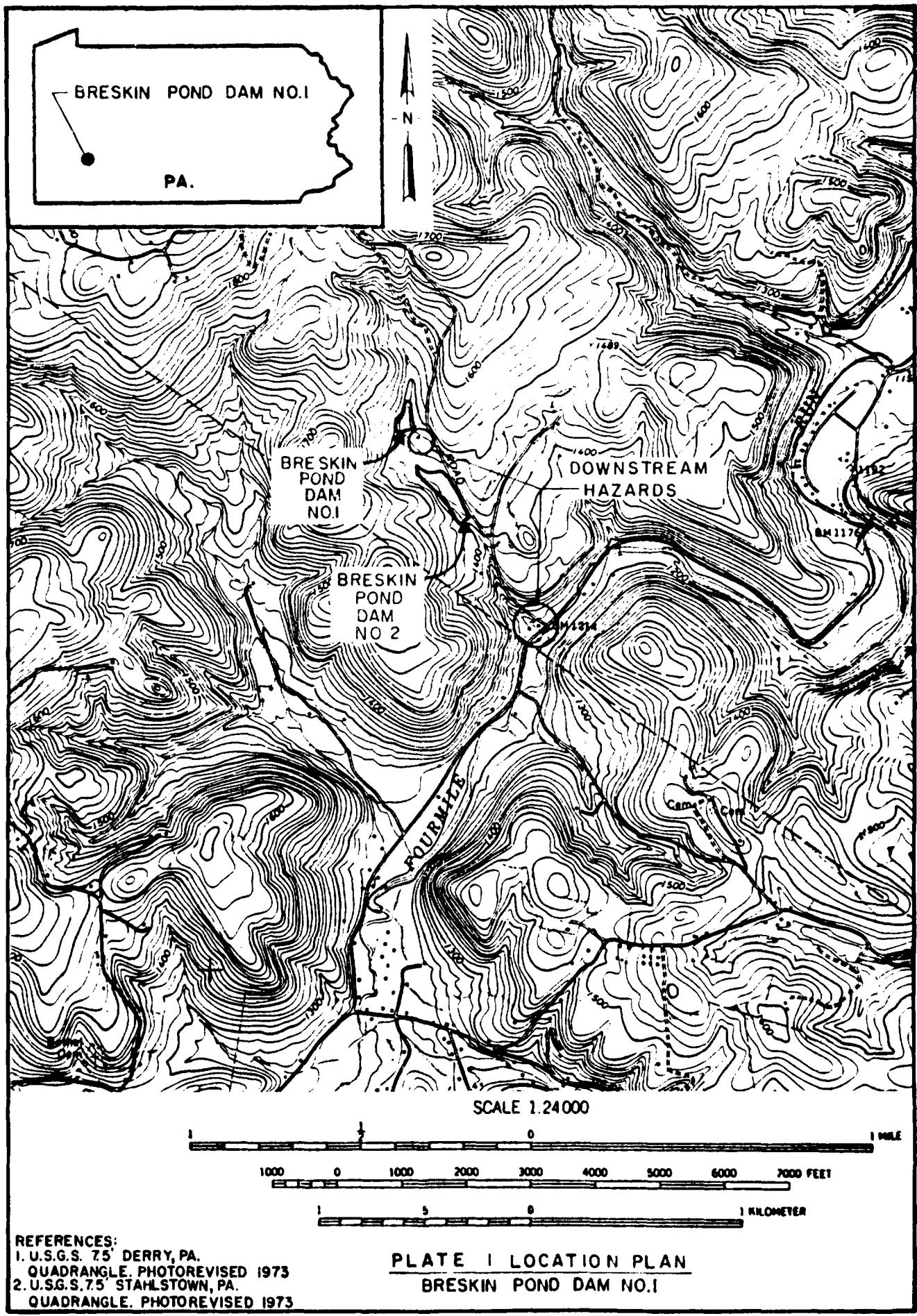
CONTENTS

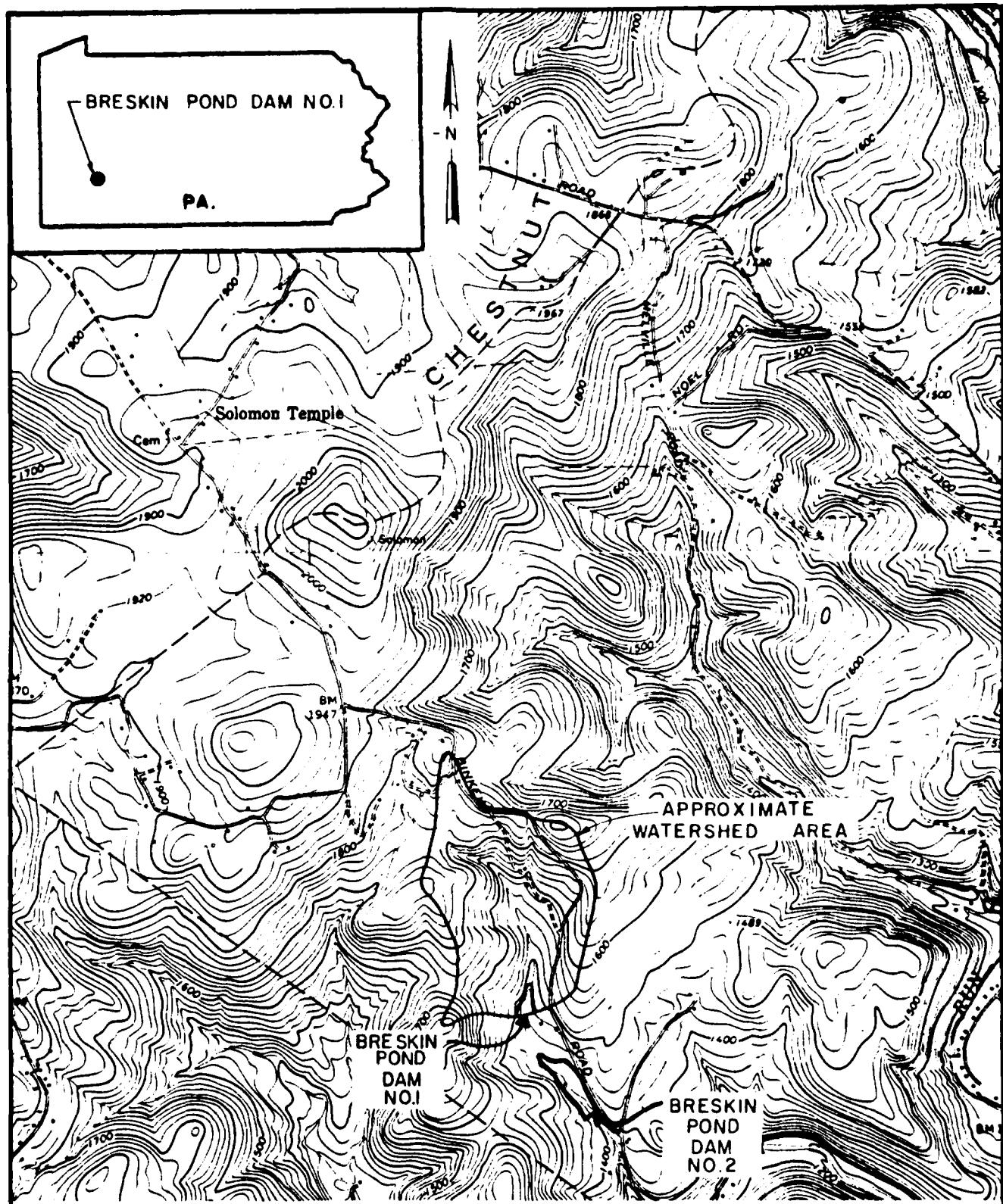
Plate 1 - Location Plan

Plate 2 - Watershed Map

Plate 3 - Field Sketch From Visual Inspection

Plate 4 - Top of Dam Profile and Typical Cross-Section
From Visual Inspection





REFERENCES:

1. U.S.G.S. 7.5' DERRY, PA.
QUADRANGLE. PHOTOREVISED 1973
2. U.S.G.S. 7.5' STAHLSTOWN, PA.
QUADRANGLE. PHOTOREVISED 1973

PLATE 2 WATERSHED MAP

BRESKIN POND DAM NO.1

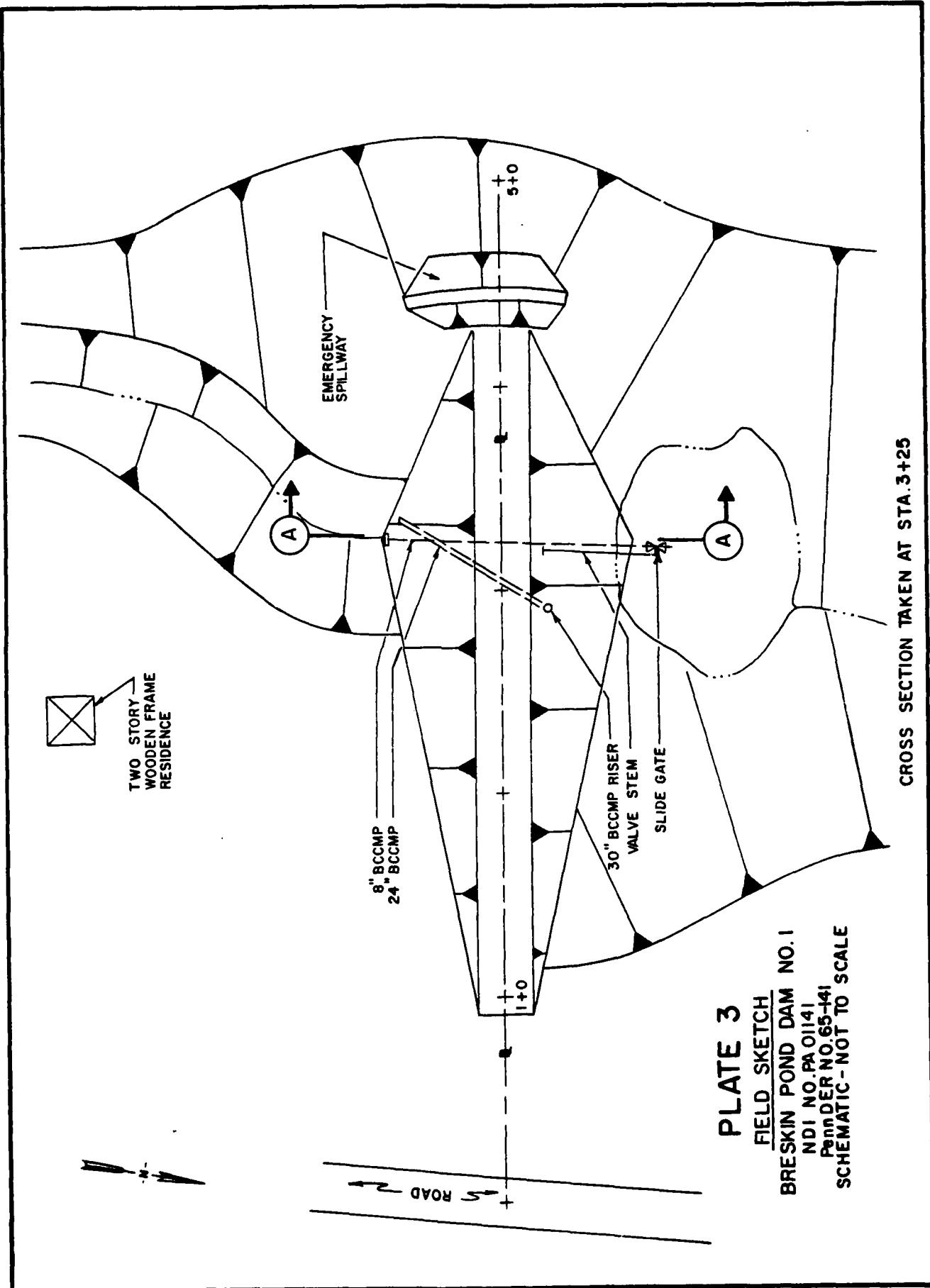


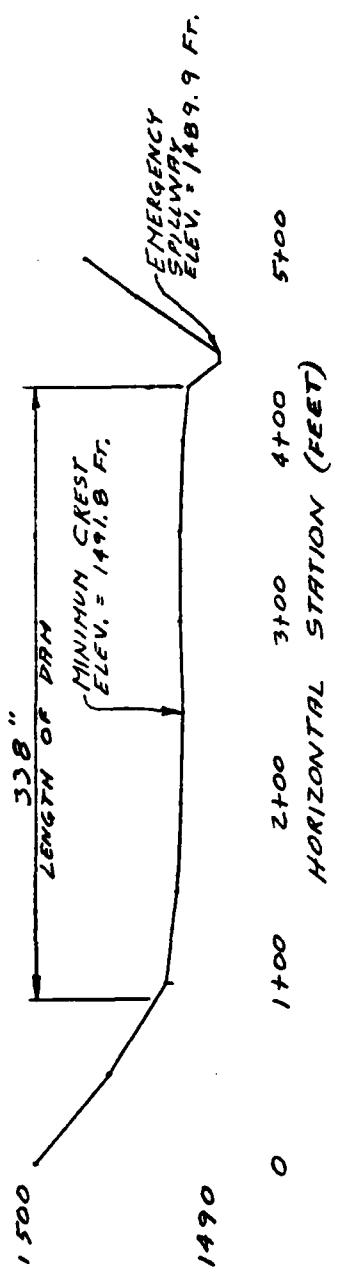
PLATE 3
FIELD SKETCH
BRESKIN POND DAM NO. 1
NDI NO. PA 0141
Pond No. 68-141
SCHEMATIC - NOT TO SCALE

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Top Of Dam Profile (looking downstream)
LENGTH OF DAM = 330 FEET



Triangular Cross Section At Sta. 3+25

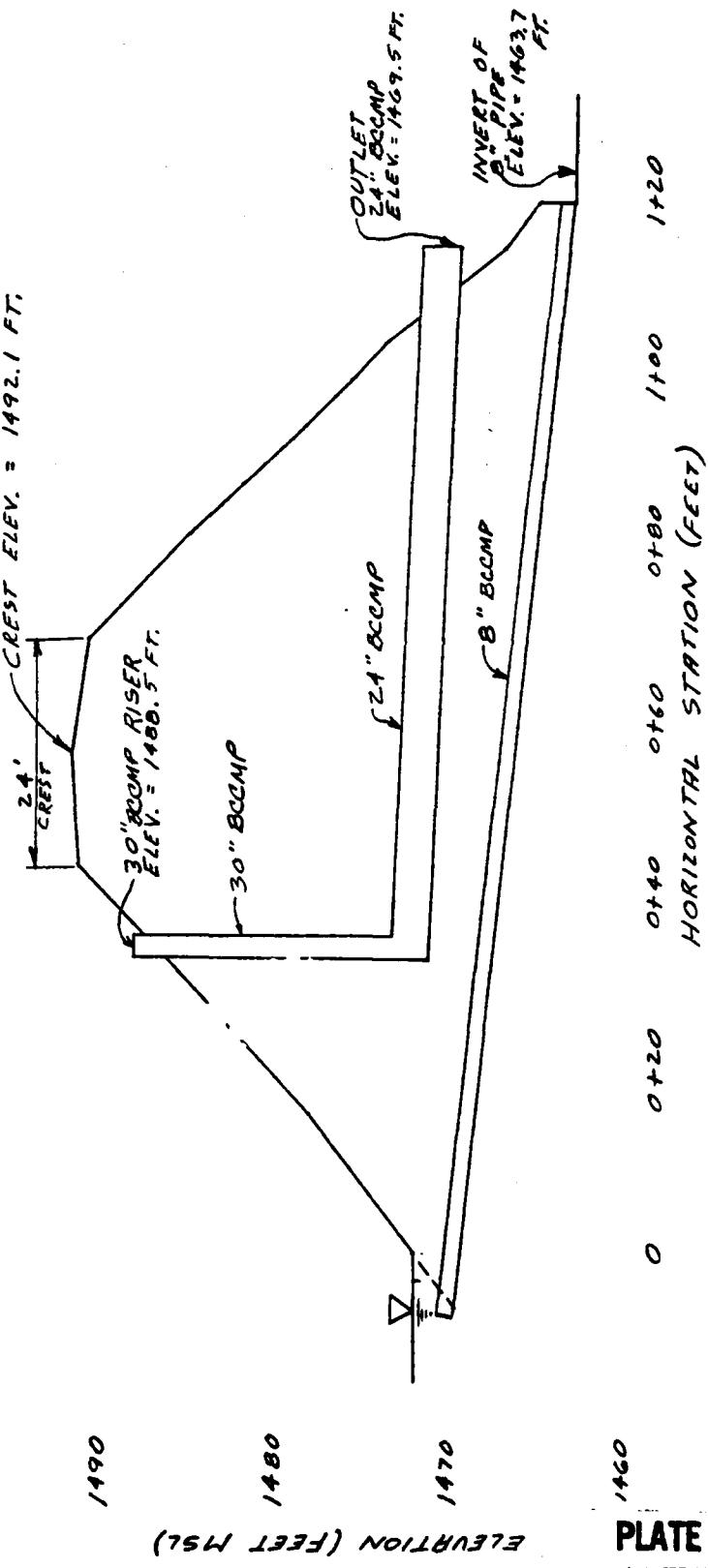


PLATE 4

APPENDIX F
REGIONAL GEOLOGY

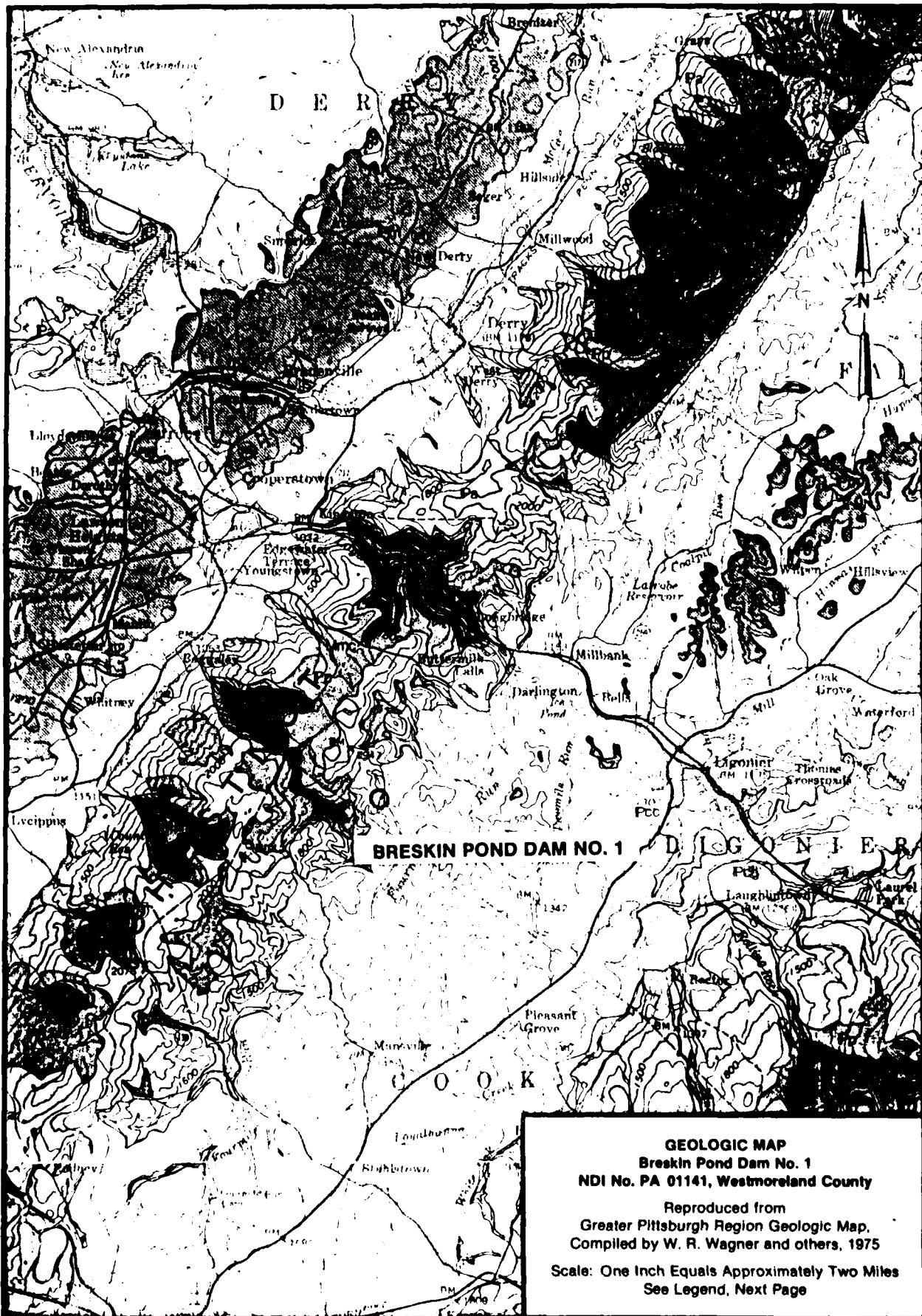
Breskin Pond Dam No. 1
NDI No. PA 01141, PennDER No. 65-141

REGIONAL GEOLOGY

Breskin Pond Dam is located in the Allegheny Mountains section of the Appalachian Plateaus physiographic province on the eastern flank of Chestnut Ridge. The dam is situated in a small valley with 10%-20% slopes in an area that shows a maximum relief of approximately 400 feet. This valley drains southwest to Fourmile Run and then north to Loyalhanna Creek.

According to the Soil Conservation Service's Soil Survey for Westmoreland County, the soil in the vicinity of the dam consists primarily of silt loams that have a Unified Soil Classification of ML-CL. These soils are colluvium derived from shales of the Freeport Formation and range from 2 to 20 feet thick. The soils may contain minor amounts of sand where underlain by sandstone members and may contain up to 25% parent material fragments in localized areas and at greater depths in the soil profile. A test boring for Joseph Breskin (boring #5) indicates 15 feet of silty sand below the center of the dam.

Geologic references indicate that bedrock in the vicinity of the dam consists of members of the Freeport Formation in the Allegheny Group. This Pennsylvanian Age Formation lies stratigraphically below the Upper Freeport Coal and is composed primarily of shale with some mineable coals and non-persistent sandstones. The coals generally have clays and limestones associated with them but have not been mined, except for localized stripping for private use. The test boring indicates that bedrock below the center of the dam is gray shale, probably of the Freeport Formation. Due to the extremely erratic occurrence of sandstone members in the formation, it could not be determined if the impoundment is founded on the Butler sandstone, a clean, permeable, sandstone found in a stratigraphically equivalent location further to the southwest. However, the possibility of infiltration problems in the impoundment area does exist. The dam is situated on the eastern flank of the Chestnut Ridge Anticline which plunges about 3° to the northeast in the vicinity of the dam. Thus, the strata beneath the dam is dipping approximately 10° to the southeast. In view of this regional dip, artesian conditions could exist beneath the dam if a suitable, confined aquifer were present.



GEOLOGY MAP LEGEND

GROUP FORMATION

DESCRIPTION

Alluvium		Qt.	Sand, gravel, clay.
Terrace deposits			Sand, clay, gravel on terraces above present rivers, includes Carmichaels Formation.
DUNKARD	Greene		Cyclic sequences of sandstone, shale, red beds, thin limestones and coals.
	Washington	Pw	Cyclic sequences of sandstone, shale, limestone, and coal; contains Washington coal bed at base.
	Waynesburg		Cyclic sequences of sandstone, shale, limestone and coal; contains Waynesburg coal bed at base.
MONONGAHELA		Pm	Cyclic sequences of shale, limestone, sandstone and coal; contains Pittsburgh coal bed at base.
CONEMAUGH	Casselman	Pcc	Cyclic sequence of sandstone, shale, red beds and thin limestone and coal.
	Ames		
	Glenshaw	Pcg	Cyclic sequences of sandstone, shale, red beds and thin limestone and coal, several fossiliferous limestone; Ames limestone bed at top.
ALLEGHENY		Pa	Cyclic sequences of shale, sandstone, limestone, and coal, contains Brookville coal at base and Upper Freeport coal at top; within group are the commercial Vanport limestone and Kittanning and Clarion coals.
		Pa	
POTTSVILLE		Pp	Sandstone and shale, contains some conglomerate and locally mineable coal.
Mauch Chunk			Red and green shale with some sandstone; contains Wymps Gap and Loyalhanna limestones.
Pocono			Sandstone and shale with Burgoon sandstone at top.

**DATE
TIME**